THE STATE OF NEW HAMPSHIRE

SITE EVALUATION COMMITTEE

DOCKET NO. 2010-01

RE: GROTON WIND, LLC

EXHIBIT CONTAINING POST-CERTIFICATE CORRESPONDENCE BETWEEN GROTON WIND, LLC AND NEW HAMSHIRE DEPARTMENT OF ENVIRONMENTAL SERVICES

March 14, 2014

1) Cover letter and attachments from Vanasse Hangen Bustlin, Inc. (Peter J. Walker) to Rene Pelletier, Assistant Director, Water Division, NH DES. (Nov. 10, 2011)

2) Electronic mail correspondence between Peter J. Walker (VHB) and Craig Rennie (NH DES) regarding a meeting between Groton Wind and DES. (Nov. 3, 2011 & Nov. 7, 2011)

3) Electronic calendar notices (2) regarding meeting at DES Office on Thursday, Nov. 20, 2011.

4) Electronic mail from Craig Rennie (DES) to Peter Walker (VHB) with Wetlands Permit Amendment and Alteration of Terrain Bureau Permit Amendment. (Dec. 5, 2011)

5) Electronic mail correspondence between Michael Leo (VHB) and Craig Rennie (DES). (Feb. 15, 2012 & Feb. 28, 2012)

(1)

Transportation
Land Development
Environmental
Services

VHB *Vanasse Hangen Brustlin, Inc.*

VIA HAND DELIVERY
November 10, 2011

Ref: 52036.00

Rene Pelletier
Assistant Director, Water Division
NH Department of Environmental Services
P.O. Box 95
29 Hazen Drive
Concord, NH 03302-0095

Re: Groton Wind Farm, Revised Site Plans
SEC Docket No. 2010-01
Wetlands File No. 2010-00745
Alteration of Terrain Permit No. 100325-033

Dear Mr. Pelletier:

On behalf of our client, Groton Wind, LLC, Vanasse Hangen Brustlin, Inc. (VHB) is pleased to submit the enclosed revised plans for the Groton Wind Farm. These plans are "Issued for Construction" and are dated October 28, 2011.

This submission is made in accordance with the NH Site Evaluation Committee's *Order and Certificate of Site and Facility with Conditions*, dated May 6, 2011, including Condition 2 of the *Wetlands Bureau Final Decision* and Condition 2 of the *Alteration of Terrain (AoT) Bureau Final Decision*.

Included with this submission please find the following materials:

- Site Plans, Groton Wind Farm dated October 28, 2011, Issued for Construction. (one copy full sized plans, one copy 11"x17" plans and one disk with PDF copy of Site Plans).
- A figure entitled *"Project Revisions"* showing eight minor changes to the project plans relative to the permitted plan set dated July 9, 2010.
- Updated Table: *"Total Impacts to Aquatic Resources"* from Wetland Permit Application
- Table: *"Summary of Impacts to Aquatic Resources"*
- Drainage Report: Addendum No. 1 (October 2011)

The Site Plans have been updated to show additional detail required for construction. In addition, eight revisions have been made to Site Plans relative to the July 9, 2010 plan set (i.e., the "Permitted Plans") as described below.

The net result of these changes is to reduce the overall footprint of the project, as defined by the limits of grading, from 115.6 acres to 103.6 acres, a reduction of about 12 acres.

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Direct wetland and stream impacts have increased, but only by a negligible amount - about 280 sq ft – from 71,680 sq ft to 71,960 sq ft. (See the enclosed impact tables.)

Each of the eight revisions is highlighted in the enclosed plan set and is further described below.

Revision 1. Operations and Maintenance Facility (Drawing C-3.1)
The proposed Operations & Maintenance (O&M) Facility has been revised. The O&M facility was moved from the east side of Clark Brook to the west side of Groton Hollow Road to a location where a fenced in switch yard was previously proposed. In addition, the total area for the O&M building and adjacent storage area has been reduced. Refer to Drawing C-3.1 for a plan showing the revised layout of the O&M area and to the enclosed Drainage Report Addendum for updated drainage calculations. This change has several environmental advantages:

- Eliminates the need for crossing Clark Brook to access the O&M area,
- Reduces the required land disturbance for the O&M area by approximately 0.7 acre, and
- Pursuant to AoT Condition 19, reduces the proposed clearing within a 50 ft buffer to two perennial streams (i.e., Clark Brook and Stream OM100) by approximately 8,150 sq ft (July 9, 2010 plans show approximately 12,400 sq ft of clearing within the buffer, while the revised plans require only 4,250 sq ft).

During the field review of the revised O&M location for any additional wetlands, a wetland was identified (GH47) that had been previously covered by debris. This wetland area will be impacted by the proposed alignment of Groton Hollow Road and increases the wetland impact at this location by 500 sq ft (See Impact #128, Drawing C-3.1).

The stormwater management design for the relocated O&M Facility is based on the stormwater BMPs that were proposed for the original location and approved by the NHDES through the SEC process. Stormwater treatment will continue to be provided by a filtration basin (with sediment forebay) as previously proposed. The filtration basin will also serve to help maintain peak runoff flow rates from the O&M building area.

Revision 2. Overhead Electric Transmission Interconnect (Drawing C-2.9, C-2.10 and C-3.1)
The overhead electrical transmission interconnect is now shown to run cross-country from the O&M Facility to NH 25. Refer to Drawings C-2.9, C-2.10 and C-3.1 for location of overhead transmission line from the O&M Facility to NH 25. As you know, the electricity from the wind farm will be transmitted to the grid along existing right-of-way except for a small portion from the site to existing NH Electric Cooperative poles that run along NH 25. Previously, the overhead line had been proposed to run north along Groton Hollow Road, but is now planned to run cross-country to NH 25 along property owned by Green Acre Woodlands, Patricia and Frederick Langford and Christopher and Therese Sheehan. This change was presented and approved as part of

the SEC proceedings and was proposed to decrease the impact along the residential section of Groton Hollow Road.

There are no direct wetland impacts associated with this change. However, clearing for the line will result in three clearing impacts of 3,050 sq ft to wetlands IC-1; IC-6 and IC-8 (Impacts #129-131).

Revision 3. East Ridge Overhead Electric Collector Line (Drawings C-2.6, C-5.1, C-5.2, C-5.3 and C-5.4)
A short segment of the overhead transmission line heading from Turbine E2 to the O&M Facility has been adjusted. This revision shortens the route and therefore decreases the amount of clearing on the East Ridge.

Revision 4. East Ridge Access Road (Drawings C-2.6, C-5.1, C-5.2, C-5.3 and C-5.4)
The access road and overhead electric line on the East Ridge has been revised. Previously, there were separate access roads to Turbines E4 and E5 and to Turbines E2 and E3. The access for the East Ridge has been combined into a single road that will reduce the length of road required to access these turbines. Turbines E2 and E3 were relocated very slightly as part of these changes. The new location of Turbine E3 and the revised road layout reduces the overall project footprint in this area, but does increase clearing within Vernal Pool ERVP1 and its buffer from 10,320 sq ft to 20,200 sq ft (See Impact #80, Sheet C-5.3). Other changes to the access road in the vicinity of Turbine E6 have resulted in a reduction in impacts to Wetland ER38 from 820 sq ft to 470 sq ft (See Impact # 73, Sheet C-5.1).

Revision 5. Groton Hollow Road from Sta. 168+00 to 174+00 (Drawings C-3.8 and C-3.9)
Approximately 600 linear feet of Groton Hollow Road has been realigned to avoid an archaeologically-sensitive area. The location of Groton Hollow Road from Sta. 168+00 to Sta. 174+00 has been modified to go around the west side of an existing old stone foundation. The road had been previously aligned around the east side of the old stone foundation. This change was presented to SEC during review their process. This change does not result in any additional disturbance area and does not require any change in wetland impacts.

Revision 6. Turbine W2 Elevation Change (Drawing C-7.4)
The elevation of Turbine W2 has been lowered approximately 9 feet to ensure the turbine base foundation will be on bedrock. This change requires a minor change in the profile of a short segment of the access road which result in additional impact of 80 sq ft (38 linear feet) to Intermittent Stream WR106B (See Impact #104, Sheet C-7.4).

Revision 7. West Ridge Overhead Electric Transmission Line (Drawing C-7.3)
Approximately 700 linear feet of overhead transmission line has been relocated east of Turbine W1 to move the overhead line further away from the turbine location. This change in the location of the overhead line completely removes the previous clearing impact of 830 sq ft to Wetland WR5 (Impact #122 is now 0 sq ft).

Revision 8. Stone Mattress Revisions
Adjustments to several stone mattress locations have been made. Stone mattresses at roadway low points have been replaced with culverts and stone mattresses have been removed in areas where very little flow was being conveyed. Attached is updated culvert spreadsheet indicating location and design flow for each culvert on project site. Stone mattress or culvert changes do not affect overall runoff (pre vs. post) discharge rates from the site. The concept of maintaining existing flow patterns in the post development condition has been retained and only one of these changes resulted in an additional wetland impact (sees Impact #120, Wetland NWR11, Drawing C-9.2). Stormwater calculations are also attached for the revised Operations and Maintenance area.

Relative to the potential effects on site drainage, none of the revisions significantly alter the Time of Concentration (Tc), Curve Number (CN) or size of the Subcatchment Areas previously analyzed. When combined with the fact that changes reduce the project footprint by about 12 acres, we determined that these changes have no significant impact on peak runoff rates, and the conclusions in our original Drainage Report remain valid. We have, however, included an addendum to the Drainage Report to update information relative to the Operation and Maintenance Facility.

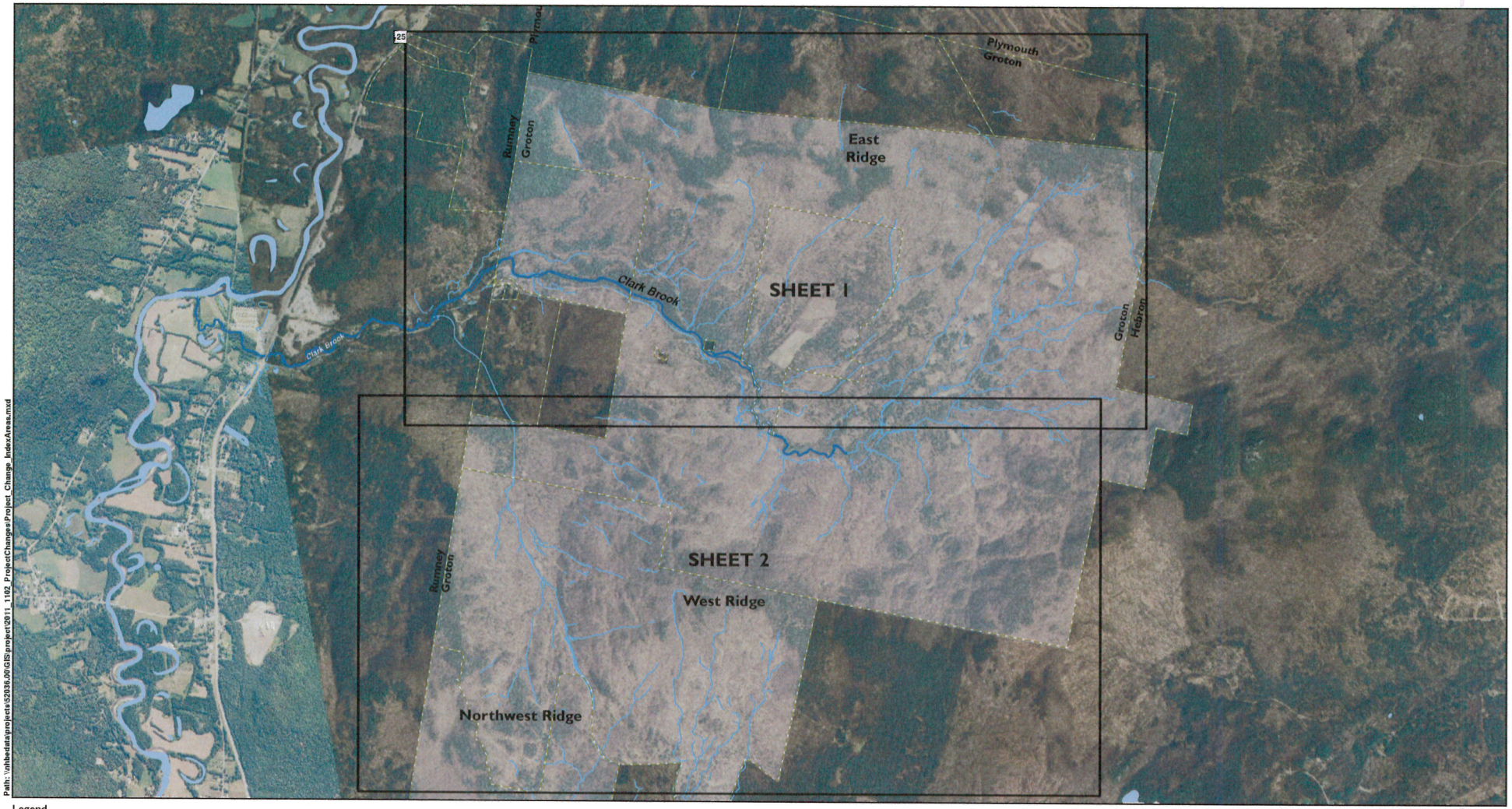
As you know, construction of the site has commenced with clearing operations underway as of last week. Iberdrola Renewables is therefore anxious to resolve any questions the Department might have regarding these plan revisions. We have already met with Craig Rennie to review these issues, and I will be contacting you to arrange for a meeting to review the project in the near future. In the meantime, as always, please don't hesitate to contact me if you have any questions or comments regarding the Groton Wind Farm project.

Very truly yours,

VANASSE HANGEN BRUSTLIN, INC.

Peter J. Walker
Director, Environmental Services

cc: Craig Rennie, NHDES
Doren Emmett, Iberdrola
Ed Cherian, Iberdrola
Jebby Varughese, Iberdrola
Mike Leo, VHB
Nancy Rendall, VHB



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Legend

- Property Boundary
- Project Study Area - Shaded in White
- River/Stream (based on USGS, Aerial Survey, Field Delineation)

0 2,000 4,000 Feet

VHB Vanasse Hangen Brustlin, Inc.

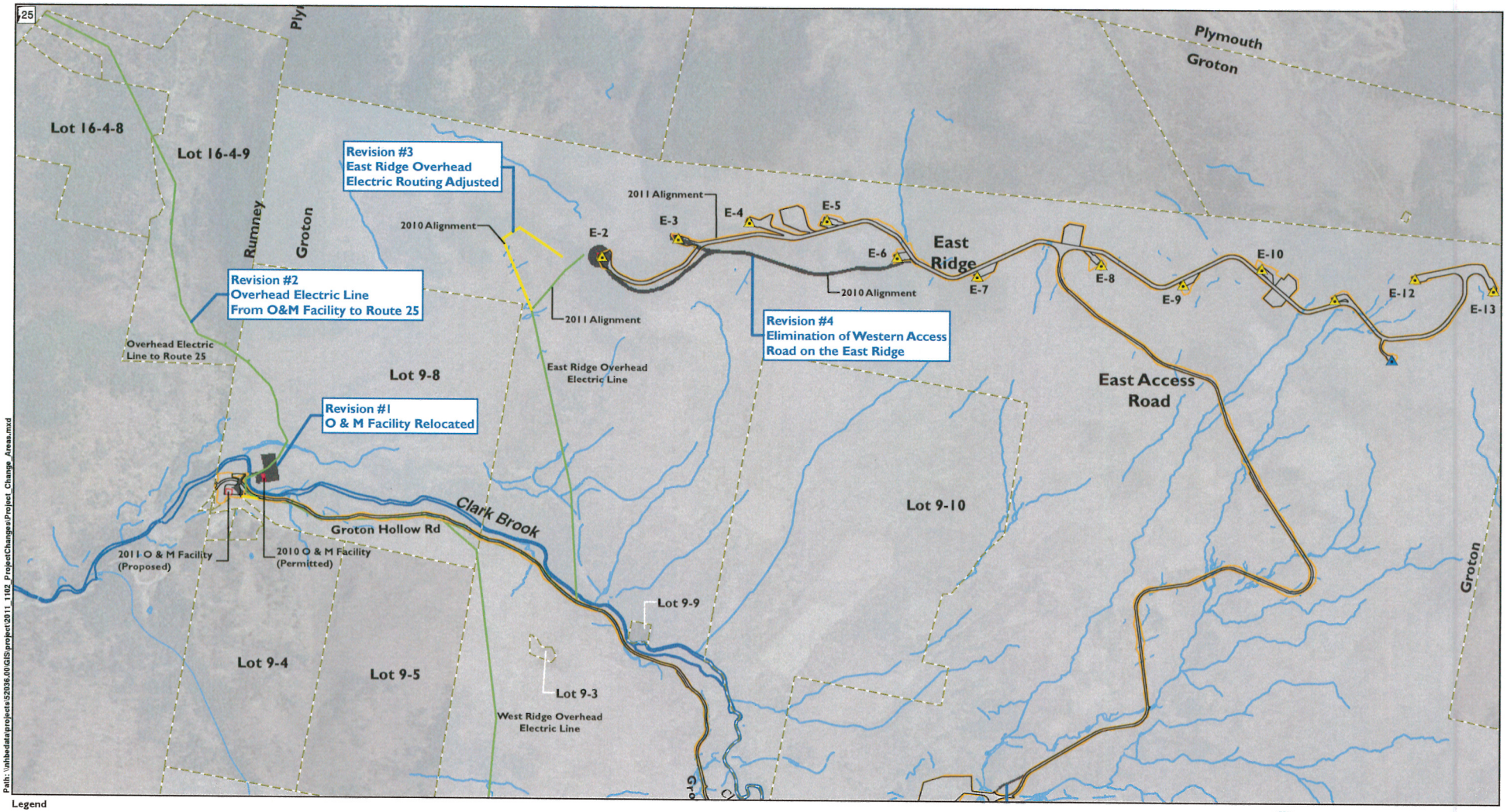
Project Revisions

Index Sheet

Groton Wind, LLC

Groton Hollow Road
Groton, NH

IBERDROLA RENEWABLES



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Legend

- Propsed Turbine
- Proposed Met Tower

October 2011 Construction Layout

- Proposed Building
- Proposed Gravel Areas
- Proposed Overhead Wire
- Proposed Clearing Line

Design Changes from the July 2010 Road Alignment

- July 2010 Overhead Line Layout
- July 2010 O & M Building Facility
- July 2010 Edge of Gravel

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Note:
The intent of this figure is to show changes from the July 9, 2010 Site Plan to October 28, 2011 Site Plans for the Groton Wind Farm.

2010 aerial photography provided by NHDOT.

0 1,000 2,000 Feet

VHB *Vanasse Hangen Brustlin, Inc.*

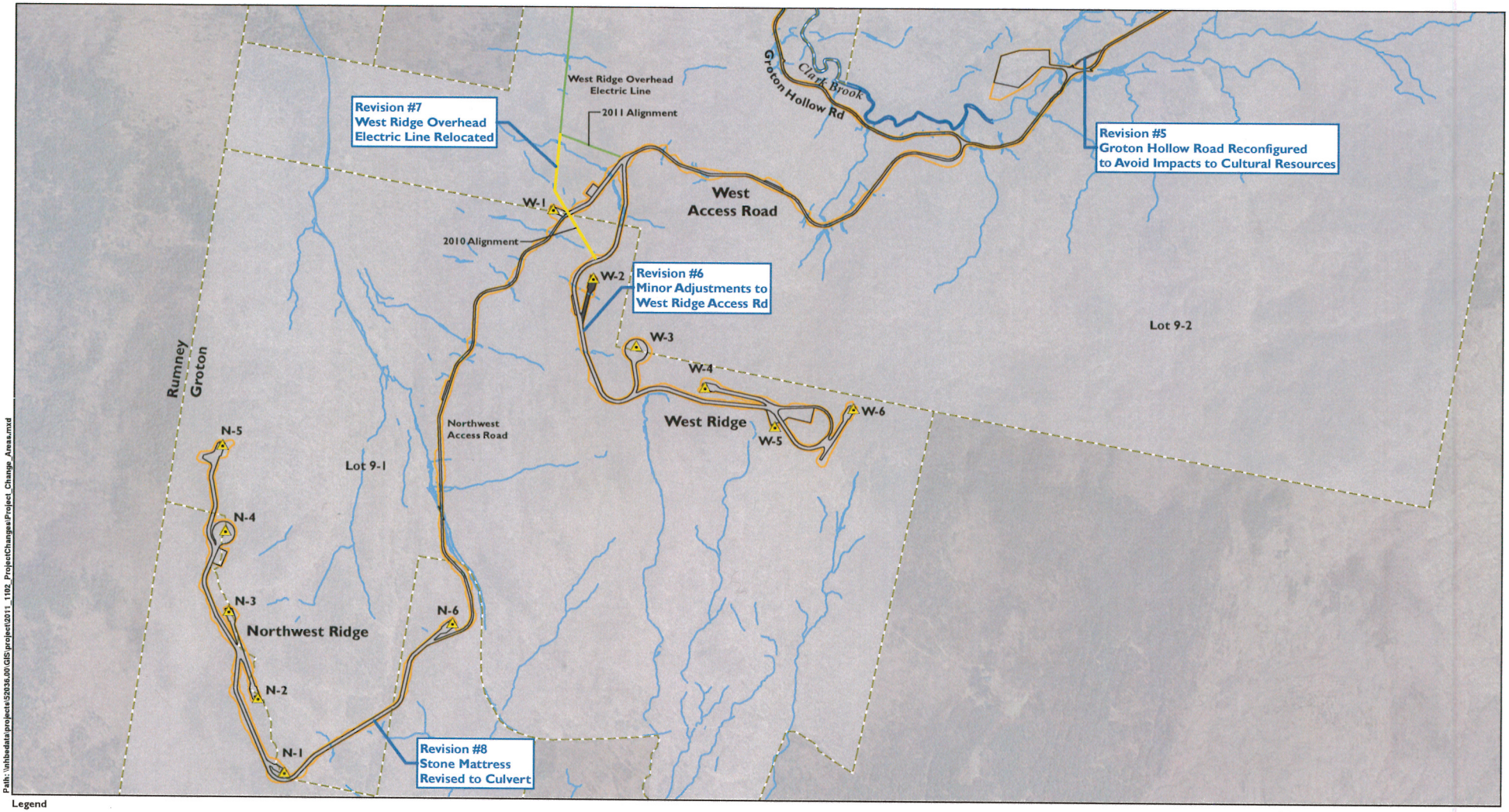
Project Revisions

Sheet 1 of 2

Groton Wind, LLC

Groton Hollow Road
Groton, NH

IBERDROLA RENEWABLES



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2010 aerial photography provided by NHDOT.

0 1,000 2,000 Feet

VHB *Vanasse Hangen Brustlin, Inc.*

Project Revisions

Sheet 2 of 2

Groton Wind, LLC

Groton Hollow Road
Groton, NH

IBERDROLA RENEWABLES

Groton Wind Farm - Summary of Impacts, November 2011

Type of Impact	2010 Impacts (SF)	2011 Impacts (SF)	Net Change Area	2010 (LF)	2011 (LF)	Net Change Linear
Direct Impacts						
Wetland Impacts - Groton Hollow Road	7,050	7,550	500	NA	NA	NA
Wetland Impacts - East Ridge	24,270	23,920	-350	NA	NA	NA
Wetland Impacts - West Ridge	7,760	7,760	0	NA	NA	NA
Wetland Impacts - Northwest Ridge	6,310	6,360	50	NA	NA	NA
Subtotal Wetland Impacts	45,390	45,590	200	NA	NA	NA
Intermittent Stream Impacts - GHR	5,290	5,290	0	1,917	1,917	0
Intermittent Stream Impacts - New Culverts	8,330	8,410	80	2,742	2,780	38
Subtotal Intermittent Stream Impacts	13,620	13,700	80	4,659	4,697	38
Perennial Stream Impacts - GHR	2,160	2,160	0	179	179	0
Perennial Stream Impacts - New Culverts	10,510	10,510	0	715	715	0
Subtotal Perennial Stream Impacts	12,670	12,670	0	894	894	0
TOTAL DIRECT JURISDICTIONAL IMPACTS	**71,680**	**71,960**	**280**	**5,553**	**5,591**	**38**
Indirect Impacts						
Wetland Clearing Impacts - Turbines	12,790	12,790	0	NA	NA	NA
Wetland Clearing Impacts - Overhead Lines	3,740	3,740	0	NA	NA	NA
Upland Buffer Clearing Vernal Pools	31,610	41,490	9880	NA	NA	NA
TOTAL INDIRECT IMPACTS	**48,140**	**58,020**	**9,880**			

Total Impacts to Aquatic Resources
(Includes VP Upland Buffer Impacts)

November 2011 Submittal

Impact Area ID[1]	Wetland ID	NWI Code	Crossing Length	2011 Impact Area (SF)[2]	2010 Permitted Impact Area (SF)[3]	Type of Resource	Type of Impact	Reason for Change
1	GH-128	R3RB1H	10	80	80	Perennial	Upgrade Culvert	NA
2	GH-127A	R4SB3E	108	220	220	Intermittent	Upgrade Culvert	NA
3	GH-127B	R4SB3E	16	50	50	Intermittent	Upgrade Culvert	NA
4	GH-125	R4SB3C	80	280	280	Intermittent	Upgrade Culvert	NA
5	GH-124A	R4SB3E	45	90	90	Intermittent	Upgrade Culvert	NA
6	GH-124B	R4SB3E	10	20	20	Intermittent	Upgrade Culvert	NA
7	GH-122	R4SB3C	330	780	780	Intermittent	Upgrade Culvert	NA
8	GH-120	R4SB3C	83	250	250	Intermittent	Upgrade Culvert	NA
9	GH-119	R2UB1H	11	80	80	Perennial	Upgrade Culvert	NA
10	GH-35	PEM1E	0	1,040	1,040	Wetland	Road Widening	NA
11	GH-38	PFO1C	0	40	40	Wetland	Road Widening	NA
12	GH-31	PEM2E	0	70	70	Wetland	Road Widening	NA
13	GH-30	PFO1C	0	960	960	Wetland	Road Widening	NA
14	GH-118	R3UB1H	12	140	140	Perennial	Upgrade Culvert	NA
15	GH-26	PEM1E	0	70	70	Wetland	Road Widening	NA
16	GH-25	PFO1C	0	170	170	Wetland	Road Widening	NA
16A	GH-113	R4SB3C	15	30	30	Intermittent	Upgrade Culvert	NA
17	GH-23	PEM2C	0	520	520	Wetland	Road Widening	NA
18	GH-112A	R4SB3E	21	100	100	Intermittent	Upgrade Culvert	NA
19	GH-112B	R4SB3E	9	80	80	Intermittent	Upgrade Culvert	NA
20	GH-21	PEM1C	0	450	450	Wetland	Road Widening	NA
21	GH-22	PEM1E	0	50	50	Wetland	Road Widening	NA
22	GH-20	PSS1F	0	570	570	Wetland	Road Widening	NA
23	GH-110A	R3UB1H	36	320	320	Perennial	Upgrade Culvert	NA
24	GH-110B	R2UB1H	23	320	320	Perennial	Upgrade Culvert	NA
25	GH-109	R4SB5C	661	1,980	1,980	Intermittent	Upgrade Culvert	NA
26	WA-1	PFO1B	0	4,180	4,180	Wetland	West Access Road	NA
27	GH-108	R3UB1H	15	190	190	Perennial	Upgrade Culvert	NA
28	GH-14	PEM1C	0	150	150	Wetland	Road Widening	NA
29	GH-107	R4SB5G	16	110	110	Intermittent	Upgrade Culvert	NA
30	GH-106 A	R4SB5E	230	710	710	Intermittent	Upgrade Culvert	NA
31	GH-106 B	R4SB5E	108	220	220	Intermittent	Upgrade Culvert	NA
32	GH-105	R3RB2H	12	100	100	Perennial	Upgrade Culvert	NA
33	GH-10	PFO1E	0	260	260	Wetland	Road Widening	NA
34	GH-104	R4SB5C	185	370	370	Intermittent	Upgrade Culvert	NA
35	GH-8	PFO1E	0	420	420	Wetland	Road Widening	NA
36	GH-7	PSS1C	0	900	900	Wetland	Road Widening	NA
37A	GH103C	R2UB1H	20	320	320	Perennial	Upgrade Bridge[4]	NA
37B	GH-4	PSS1C	0	1,380	1,380	Wetland	Road Widening	NA
38	EA-103	R2UB1H	40	610	610	Perennial	Upgrade Culvert	NA
39	EA-6	PUBE	0	650	650	Wetland	East Access Road	NA
40	EA-104	R4SB5E	54	110	110	Intermittent	New Culvert	NA
41	EA-10	PFO1C	0	1,230	1,230	Wetland	East Access Road	NA
42	EA-105	R4SB3E	193	390	390	Intermittent	New Culvert	NA
43	EA-106	R4SB3E	95	190	190	Intermittent	New Culvert	NA
44	EA-107	R3UB1H	63	810	810	Perennial	New Culvert	NA
45	EA-16	PEM2E	0	80	80	Wetland	East Access Road	NA
46	EA-108	R3UB1H	40	350	350	Perennial	New Culvert	NA
47	EA-109	R4SB7F	54	160	160	Intermittent	New Culvert	NA
48	EA-110-B	R4SB7E	65	250	250	Intermittent	New Culvert	NA
49	EA-110-A	R4SB7E	16	130	130	Intermittent	New Culvert	NA
50	EA-17	PEM2E	0	320	320	Wetland	East Access Road	NA
51	EA-18	PFO1C	0	550	550	Wetland	East Access Road	NA
52	EA-111	R3UB1H	105	1,410	1,410	Perennial	New Culvert	NA
53	EA-19	PSS1C	0	690	690	Wetland	East Access Road	NA
54	EA-21	PSS1C	0	560	560	Wetland	East Access Road	NA
55	EA-22	PSS1E	0	3,210	3,210	Wetland	East Access Road	NA
56	EA-24	PSS1E	0	2,700	2,700	Wetland	East Access Road	NA
57	EA-114	R3RB2H	75	640	640	Perennial	New Culvert	NA
58	EA-25	PSS1B	0	2,840	2,840	Wetland	East Access Road	NA
59	EA-26	PFO1E	0	1,600	1,600	Wetland	East Access Road	NA
60	EA-116	R4SB5E	30	120	120	Intermittent	New Culvert	NA
61	EA-28	PSS1B	0	250	250	Wetland	East Access Road	NA
62	EA-117	R4SB5E	91	360	360	Intermittent	New Culvert	NA
63	EA-118	R4SB1E	91	720	720	Intermittent	New Culvert	NA
64	EA-119	R4SB3E	60	480	480	Intermittent	New Culvert	NA
65	EA-30	PSS1C	0	100	100	Wetland	East Access Road	NA
66	EA-120	R4SB2E	62	250	250	Intermittent	New Culvert	NA
67	EA-33	PSS1E	0	70	70	Wetland	East Access Road	NA
68	EA-122	R4SB7E	91	360	360	Intermittent	New Culvert	NA
69	ER-107 A	R4SB5E	159	480	480	Intermittent	New Culvert	NA
70	ER-33	PFO1E	0	210	210	Wetland	East Ridge Road	NA
71	ER-107 B	R4SB5E	73	150	150	Intermittent	New Culvert	NA
72	ER-35	PFO1E	0	1,040	1,040	Wetland	East Ridge Road	NA

Total Impacts to Aquatic Resources
(Includes VP Upland Buffer Impacts)

November 2011 Submittal

Impact Area ID[1]	Wetland ID	NWI Code	Crossing Length	2011 Impact Area (SF)[2]	2010 Permitted Impact Area (SF)[3]	Type of Resource	Type of Impact	Reason for Change
73	ER-38	PFO1E	0	470	820	Wetland	East Ridge Road[6]	Reduction of 350 sq ft due to reconfiguration of East Ridge Access Road
74	ER-39A	PFO1E	0	2,200	2,200	Wetland	East Ridge Road	NA
75	ER-39B	PFO1E	0	2,070	2,070	Wetland	Clearing for Turbine[5]	NA
76	ER-109	R4SB5F	56	220	220	Intermittent	New Culvert	NA
77	ER-42	PFO4E	0	1,370	1,370	Wetland	East Ridge Road	NA
80	ERVP1	NA	0	20,200	10,320	VP Buffer	Upland Buffer Clearing[6,7]	Minor relocation of Turbine E-3 in order to facilitate reduction in cut/fill slopes; VP Buffer clearing increases from 10,320 to 20,200
82	ER-21	PSS1E	0	1,060	1,060	Wetland	East Ridge Road	NA
83	ER-100	R4SB5E	75	170	170	Intermittent	New Culvert	NA
84	ER-101	R4SB3E	117	230	230	Intermittent	New Culvert	NA
85	ER-13	PFO1C	0	5,810	5,810	Clearing	Clearing for Turbine[5]	NA
86	ER-14	PFO1C	0	1,290	1,290	Clearing	Clearing for Turbine[5]	NA
87	ER-14	PEM1B	0	410	410	Wetland	East Ridge Road	NA
88	ER-102	R4SB5E	35	140	140	Intermittent	New Culvert	NA
89	ER-16	PFO4C	0	1,240	1,240	Wetland	Crane Pad	NA
90	ER-17	PFO4C	0	1,070	1,070	Wetland	Crane Pad	NA
91	ER-18	PSS1C	0	950	950	Clearing	Clearing for Turbine[5]	NA
92	WA-2	PSS1E	0	270	270	Wetland	West Access Road	NA
93	WR-100	R4SB2F	80	500	500	Intermittent	New Culvert	NA
94	WA-4	PSS1E	0	60	60	Wetland	West Access Road	NA
95	WR-101	R4SB5E	60	120	120	Intermittent	New Culvert	NA
96	WR-102	R2RB2H	52	830	830	Perennial	New Culvert	NA
97	WR-103	R4SB2C	171	490	490	Intermittent	New Culvert	NA
98	NWA-1	PSS1E	0	150	150	Wetland	West Access Road	NA
99	WR-104 A	R2UB1H	85	1,730	1,730	Perennial	New Culvert	NA
100	WR-104B	R2UB1H	85	1,010	1,010	Perennial	New Culvert	NA
101	WR-3	PSS1E	0	660	660	Wetland	West Ridge Road	NA
102	WR-4	PSS1E	0	310	310	Wetland	West Ridge Road	NA
103	WR-106A	R4SB3C	99	260	260	Intermittent	New Culvert	NA
104	WR-106B	R4SB3C	160	320	240	Intermittent	New Culvert[6]	Regrading required to ensure Turbine W-2 foundation is on bedrock increased impact to intermittent stream
105	ER-10	PFO1C	0	1,670	1,670	Wetland	Clearing for Turbine[5]	NA
106	WR-22	PFO1E	0	1,970	1,970	Wetland	West Ridge Road	NA
107	WR-21	PFO1E	0	960	960	Wetland	Clearing for Turbine[5]	NA
108	NWA-100	R4SB3C	87	170	170	Intermittent	New Culvert	NA
109	NWA-101	R4SB2F	71	140	140	Intermittent	New Culvert	NA
110	NWA-102	R3UB1H	80	1,280	1,280	Perennial	New Culvert	NA
111	WA-11A	PSS1A	0	160	160	Wetland	West Access Road	NA
112	NWA-103	R4SB2G	81	190	190	Intermittent	New Culvert	NA
113	NWA-105	R4SB1F	54	140	140	Intermittent	New Culvert	NA
114	NWA-106A	R4SB2F	70	140	140	Intermittent	New Culvert	NA
115	NWA-106B	R4SB2F	70	140	140	Intermittent	New Culvert	NA
116	NWA-104	R2UB3H	130	2,450	2,450	Perennial	New Culvert	NA
117	NWA-107	R4SB3C	115	230	230	Intermittent	New Culvert	NA
118	NWR-3	PFO1E	0	40	40	Wetland	Clearing for Turbine[5]	NA
119	NWR-8	PFO1E	0	5,500	5,500	Wetland	Northwest Access Road	NA
120	NWR-11	PFO1E	0	790	740	Wetland	Northwest Access Road[6]	Replaced stone mattress with culvert, required additional fill to obtain minimum cover over culvert
121	NWA-110	R4SB1E	75	220	220	Intermittent	New Culvert	NA
122	WR-5	PSS1C	0	0	0	Wetland	Clearing for Overhead Lines[5, 6]	NA
123	NWA-2	PEM1E	0	70	70	Wetland	Northwest Access Road	NA
124	WR-1A	NA	0	21,290	21,290	VP Buffer	Upland Buffer Clearing	NA
125	OHW-2	PEM1E	0	690	690	Wetland	Clearing for Overhead Lines	NA
126	EA-123	R4SB5E	70	140	140	Intermittent	New Culvert[8]	NA
127	WR-108	R4SB2C	100	300	300	Intermittent	New Culvert[8]	NA
128	GH-47	PEM2C	0	500	0	Wetland	Groton Hollow Road[6]	Wetland discovered after permitting plans were completed.
129	IC-6	PFO1E/F	0	1,440	1,440	Wetland	Clearing for Overhead Lines[6]	Revised OHE Interconnect Alignment
130	IC-8	PFO1C	0	180	180	Wetland	Clearing for Overhead Lines[6]	Revised OHE Interconnect Alignment
131	IC-1	PSS1F	0	1,430	1,430	Wetland	Clearing for Overhead Lines[6]	Revised OHE Interconnect Alignment
				129,980	119,820			

Notes:

Shaded rows represent changes since July 2010.

1. See Impact IDs as shown on the project plans.
2. As shown on plans dated October 28, 2011, Issued for Construction.
3. As shown on plans dated July 9, 2010, Issued for Permitting.
4. Temporary impact to remove existing log bridge and replace with a 17.3-foot by 3.8-foot steel box that spans the stream and banks.
5. Includes temporary wetland impacts due to clearing within the 150-foot turning radius for the proposed turbines and clearing along the footprint for proposed overhead lines.
6. These impact areas were changed since July, 2010 in response to design changes and the revised location of the overhead line to NH Route 25.
7. This table includes two upland impacts for clearing in the 100-foot vernal pool buffer that total 41,490 square feet. Direct impacts are listed separately.
8. These impact areas were added to the plan set in July, 2010 in response to NHDES Comments.

Drainage Report: Addendum No. 01

Proposed Groton Wind Farm

Groton Hollow Road
Groton, New Hampshire

Prepared for **Groton Wind, LLC**
Concord, New Haampshire

Prepared by ***VHB*/Vanasse Hangen Brustlin, Inc.**
Bedford, New Hampshire 03110

October 2011

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Summary

The following calculations are provided to supplement the Drainage Report for the Proposed Groton Wind Farm Project in Groton, New Hampshire. The updated calculations are associated with the following revisions that have been modified as part of the design and development of the project:

- Relocation of the proposed Operations & Maintenance Facility.
- Revised layout of the proposed overhead electric transmission interconnect line.
- Revised layout of the East Ridge overhead electric collector line.
- Revised layout of the East Ridge Access Road.
- Realignment of Groton Hollow Road (Sta. 168+00 to 174+00).
- Turbine W2 Elevation Change.
- Revised layout of the West Ridge overhead electric transmission line.
- Stone mattress revisions/adjustments.

The above changes help to reduce the overall footprint of the project from 115.6 acres to 103.6 acres (approximately 12 acres).

Storm Water Quantity Mitigation

A peak runoff rate comparison for existing and proposed conditions was included in the original Drainage Report for the project and concluded that the proposed project was not anticipated to significantly change the peak runoff rates for the study area. It should be noted that the changes result in a 12 acre reduction in the overall project footprint. The majority of which occur within Subcatchment Area 101 (refer to Figure 4 - Drainage Area Map (Proposed) in the original Drainage Report). Additionally, none of the revisions significantly alter the Time of Concentration (Tc), Curve Number (CN) or size of the Subcatchment Areas previously analyzed. As such, the changes did not require that the hydrologic calculations be recomputed. A copy of Table 4 (Peak Storm Water Runoff Rate Summary) from the Drainage Report is provided below:

Table 4:
Peak Storm Water Runoff Rate Summary

Discharge Point		Condition	Peak Flow for Given Storm (cfs) 2-yr	10-yr	50-yr
D1	Clark Brook	Existing	105.7	761.7	1583.8
		Proposed	105.7	762.0	1584.4
			0.0	0.3	0.6
D2	Halls Brook/box culvert under Route 25	Existing	25.5	327.4	763.8
		Proposed	25.5	327.3	763.5
			0.0	-0.1	-0.3
D3	Pond/96" CMP under Route 25	Existing	6.6	128.7	336.2
		Proposed	6.6	128.5	335.7
			0.0	-0.2	-0.5
D4	Unnamed Stream/66" RCP Culvert under Route 25	Existing	3.6	88.3	245.5
		Proposed	3.6	88.3	245.5
			0.0	0.0	0.0
D5	Unnamed Stream/11' wide bridge under Route 3A	Existing	26.5	280.9	632.7
		Proposed	26.5	280.9	632.7
			0.0	0.0	0.3
D6	Unnamed Stream/15' wide bridge under route 3A	Existing	29.5	220.6	458.8
		Proposed	29.5	220.6	458.8
			0.0	0.0	0.0
D7	Wise Brook	Existing	18.8	102.2	194.5
		Proposed	18.8	102.1	194.3
			0.0	-0.1	-0.2

Operations & Maintenance Facility

The O&M building was moved from the east side of Clark Brook to the west side of Groton Hollow Road to a location where a fenced in switch yard was previously proposed. In addition, the total area for the O&M building and adjacent storage area has been reduced. Refer to Drawing C-3.1 for a plan showing the revised layout of the O&M area. This change has several environmental advantages:

- Eliminates the need for crossing Clark Brook to access the O&M area,
- Reduces the required land disturbance for the O&M area by approximately 0.7 acre, and
- Pursuant to AoT Condition 19, reduces the proposed clearing within a 50 ft buffer to two perennial streams (i.e., Clark Brook and Stream OM100) by approximately 8,150 sq ft (July 9, 2010 plans show approximately 12,400 sq ft of clearing within the buffer, while the revised plans require only 4,250 sq ft).

The stormwater management design for the relocated O&M building is based on the stormwater BMP's that were proposed for the original location and approved by the NHDES through New Hampshire's SEC process. Stormwater treatment will continue to be provided by a filtration basin (with sediment forebay) as previously proposed. The filtration basin will also serve to help maintain peak runoff flow rates from the O&M building area.

Culvert Sizing Summary

Several of the stone mattress locations have been adjusted. Stone mattresses at roadway low points have been replaced with culverts and stone mattresses have been removed in areas where very little flow was being conveyed. The Culvert Sizing Summary (included with the original Drainage Report) has been updated to reflect these, and other, minor changes and is included in this addendum.

Project #: 52036.00
Project: Groton Wind Farm
Location: Groton, New Hampshire
Calculated by: B. Anderson Date: 11/1/11
Checked by: Date:
Title: Pre- vs Post-Development Peak Runoff Rate Comparison (O&M Building Area)

Operations & Maintenance Area

	Peak Runoff Rate (cfs)			
Condition	1-yr	2-yr	10-yr	50-yr
Existing	0.18	0.53	6.65	14.94
Proposed	0.18	0.28	4.26	14.80
Net Change	0.00	-0.25	-2.39	-0.14

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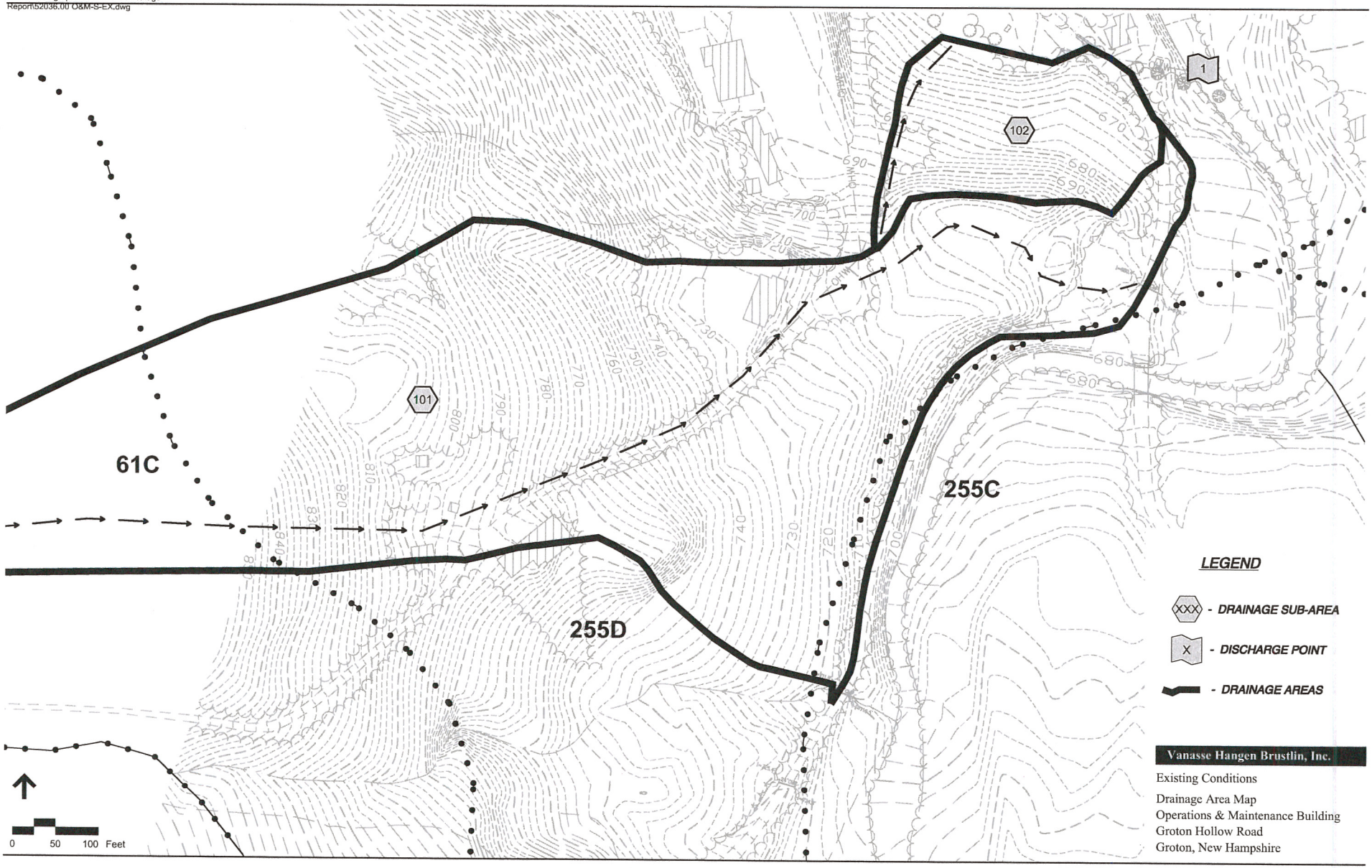

Vanasse Hangen Brustlin, Inc.

Existing Conditions

Drainage Area Map
Operations & Maintenance Building
Groton Hollow Road
Groton, New Hampshire

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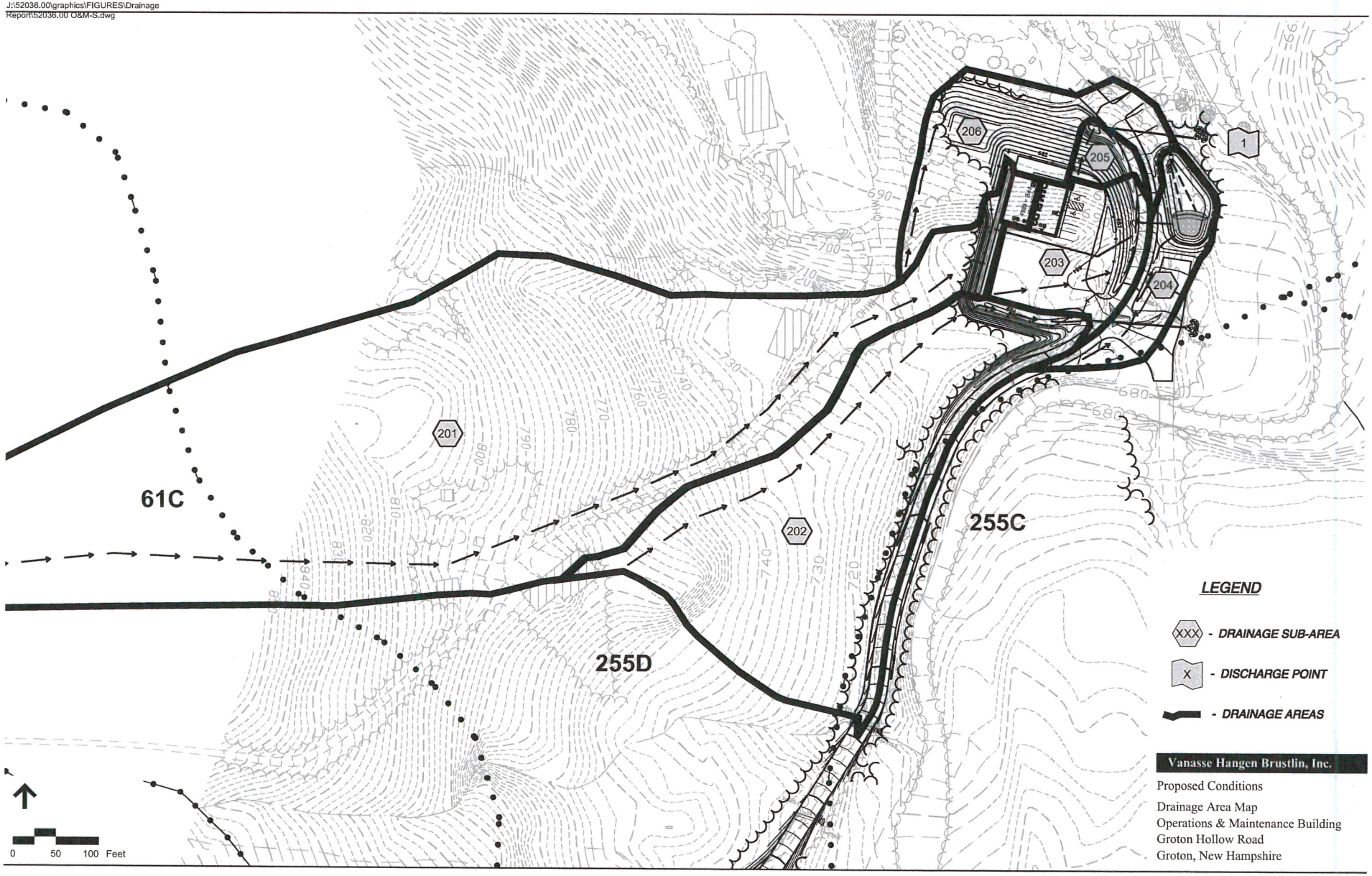
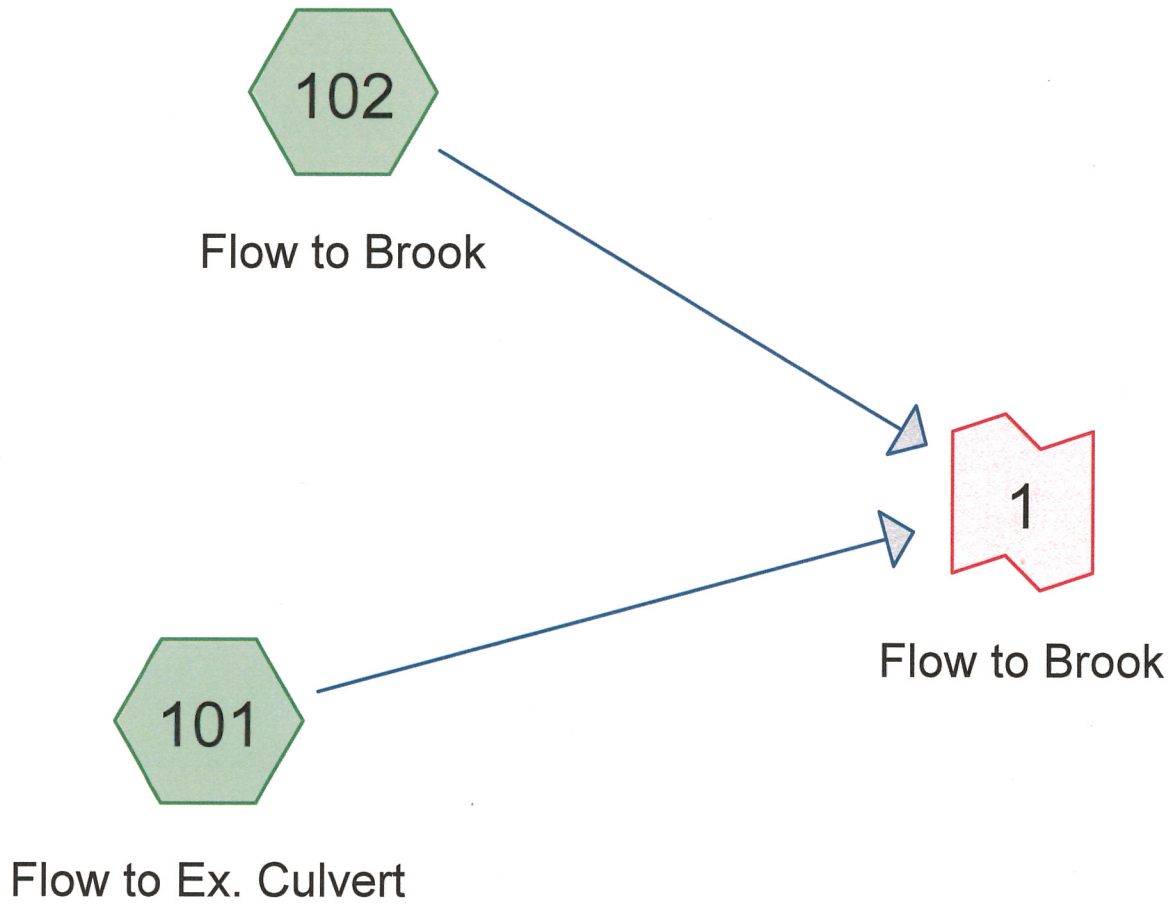


Vanasse Hangen Brustlin, Inc.

Proposed Conditions

Drainage Area Map
Operations & Maintenance Building
Groton Hollow Road
Groton, New Hampshire



Subcat Reach Pond Link

Drainage Diagram for Groton-Existing O&M 2011-10-29
Prepared by Vanasse Hangen Brustlin, Inc., Printed 11/1/2011

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
1.046	55	Woods, Good, HSG B (102)
0.085	61	>75% Grass cover, Good, HSG B (102)
10.627	67	Based on CN for E101 (101)
0.064	85	Gravel roads, HSG B (102)
11.822		**TOTAL AREA**

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101: Flow to Ex. Culvert Runoff Area=10.627 ac 0.00% Impervious Runoff Depth=0.12"
Flow Length=1,690' Tc=28.5 min CN=67 Runoff=0.18 cfs 0.104 af

Subcatchment102: Flow to Brook Runoff Area=1.195 ac 0.00% Impervious Runoff Depth=0.00"
Flow Length=240' Tc=18.2 min CN=57 Runoff=0.00 cfs 0.000 af

Link 1: Flow to Brook Inflow=0.18 cfs 0.104 af
Primary=0.18 cfs 0.104 af

Total Runoff Area = 11.822 ac Runoff Volume = 0.104 af Average Runoff Depth = 0.11"
100.00% Pervious = 11.822 ac 0.00% Impervious = 0.000 ac

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101: Flow to Ex. Culvert Runoff Area=10.627 ac 0.00% Impervious Runoff Depth=0.21"
Flow Length=1,690' Tc=28.5 min CN=67 Runoff=0.53 cfs 0.184 af

Subcatchment102: Flow to Brook Runoff Area=1.195 ac 0.00% Impervious Runoff Depth=0.01"
Flow Length=240' Tc=18.2 min CN=57 Runoff=0.00 cfs 0.001 af

Link 1: Flow to Brook Inflow=0.53 cfs 0.186 af
Primary=0.53 cfs 0.186 af

Total Runoff Area = 11.822 ac Runoff Volume = 0.186 af Average Runoff Depth = 0.19"
100.00% Pervious = 11.822 ac 0.00% Impervious = 0.000 ac

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101: Flow to Ex. Culvert Runoff Area=10.627 ac 0.00% Impervious Runoff Depth=0.91"
Flow Length=1,690' Tc=28.5 min CN=67 Runoff=6.52 cfs 0.807 af

Subcatchment102: Flow to Brook Runoff Area=1.195 ac 0.00% Impervious Runoff Depth=0.36"
Flow Length=240' Tc=18.2 min CN=57 Runoff=0.14 cfs 0.036 af

Link 1: Flow to Brook Inflow=6.65 cfs 0.843 af
Primary=6.65 cfs 0.843 af

Total Runoff Area = 11.822 ac Runoff Volume = 0.843 af Average Runoff Depth = 0.86"
100.00% Pervious = 11.822 ac 0.00% Impervious = 0.000 ac

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment101: Flow to Ex. Culvert Runoff Area=10.627 ac 0.00% Impervious Runoff Depth=1.67"
Flow Length=1,690' Tc=28.5 min CN=67 Runoff=14.32 cfs 1.479 af

Subcatchment102: Flow to Brook Runoff Area=1.195 ac 0.00% Impervious Runoff Depth=0.87"
Flow Length=240' Tc=18.2 min CN=57 Runoff=0.77 cfs 0.087 af

Link 1: Flow to Brook Inflow=14.94 cfs 1.566 af
Primary=14.94 cfs 1.566 af

Total Runoff Area = 11.822 ac Runoff Volume = 1.566 af Average Runoff Depth = 1.59"
100.00% Pervious = 11.822 ac 0.00% Impervious = 0.000 ac

Summary for Subcatchment 101: Flow to Ex. Culvert

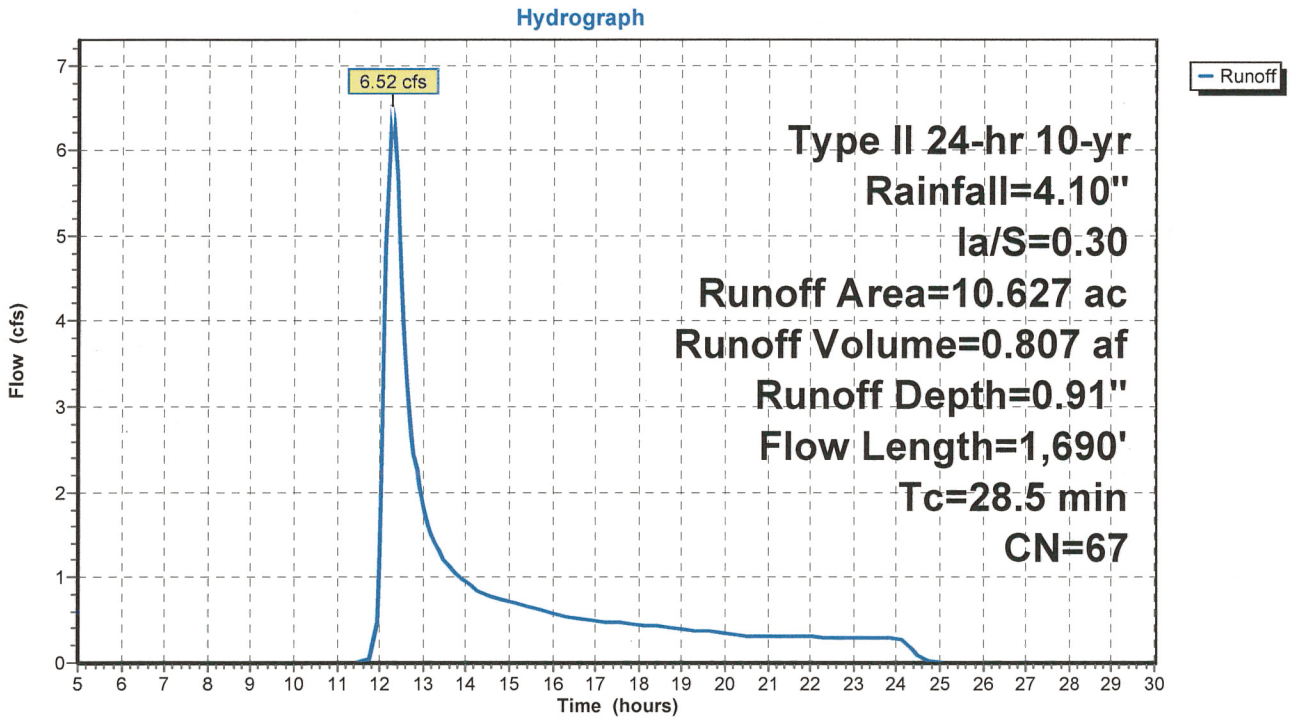
Runoff = 6.52 cfs @ 12.28 hrs, Volume= 0.807 af, Depth= 0.91"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

	Area (ac)	CN	Description
*	10.627	67	Based on CN for E101
	10.627		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.5	100	0.1500	0.09		**Sheet Flow, Woods** Woods: Dense underbrush n= 0.800 P2= 2.60"
9.3	1,400	0.2500	2.50		**Shallow Concentrated Flow, Woods** Woodland Kv= 5.0 fps
0.7	190	0.1200	4.77	14.31	**Trap/Vee/Rect Channel Flow, Drainage Way** Bot.W=5.00' D=0.50' Z= 2.0 '/' Top.W=7.00' n= 0.060
28.5	1,690	Total			

Subcatchment 101: Flow to Ex. Culvert



Summary for Subcatchment 102: Flow to Brook

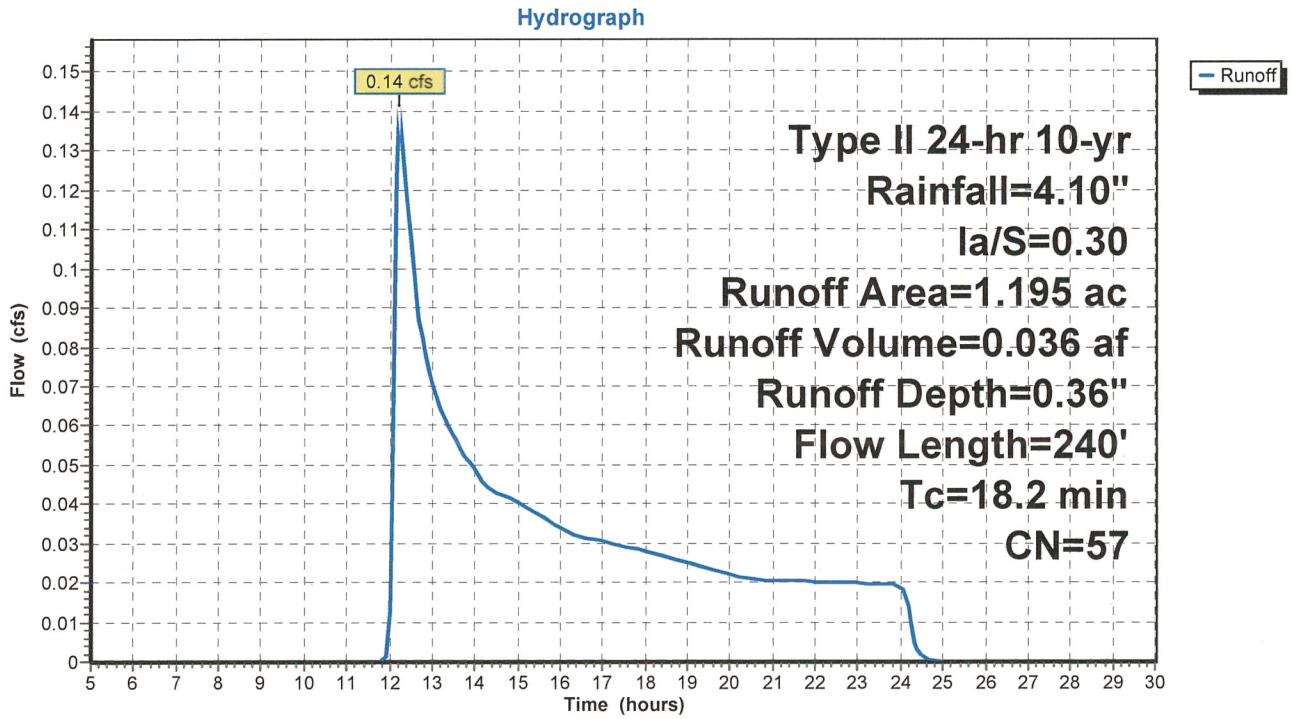
Runoff = 0.14 cfs @ 12.23 hrs, Volume= 0.036 af, Depth= 0.36"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

Area (ac)	CN	Description
0.064	85	Gravel roads, HSG B
0.085	61	>75% Grass cover, Good, HSG B
1.046	55	Woods, Good, HSG B
1.195	57	Weighted Average
1.195		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.2	100	0.1800	0.10		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.60"
1.0	140	0.2000	2.24		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
18.2	240	Total			

Subcatchment 102: Flow to Brook

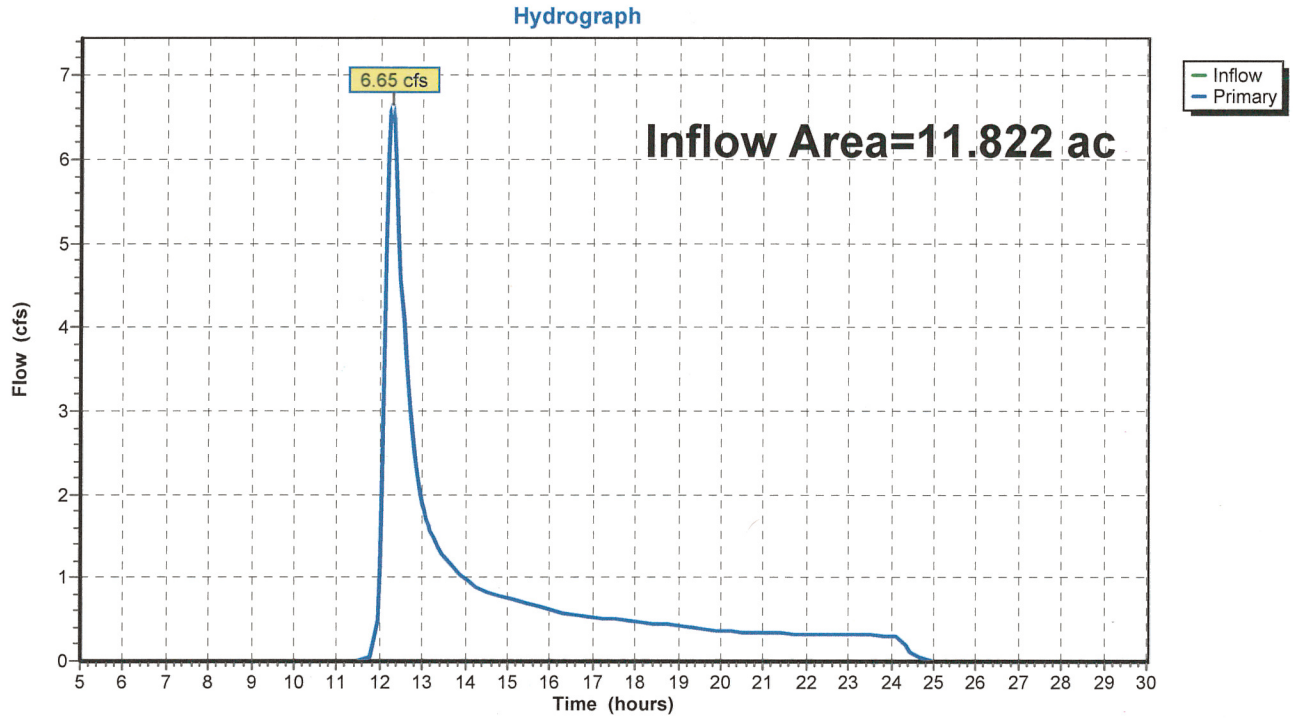


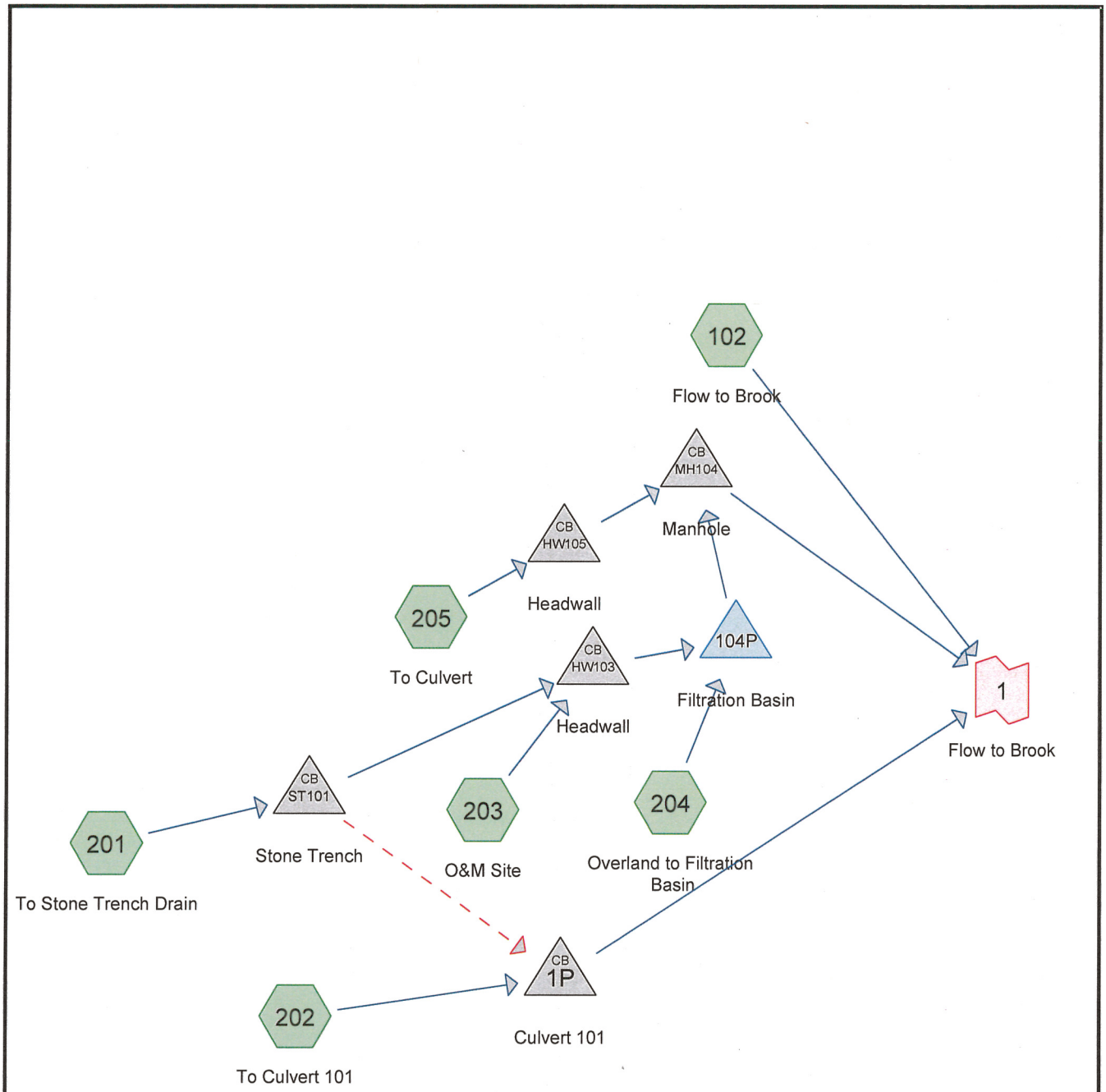
Summary for Link 1: Flow to Brook

Inflow Area = 11.822 ac, 0.00% Impervious, Inflow Depth = 0.86" for 10-yr event
Inflow = 6.65 cfs @ 12.28 hrs, Volume= 0.843 af
Primary = 6.65 cfs @ 12.28 hrs, Volume= 0.843 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs

Link 1: Flow to Brook





Subcat Reach Pond Link

Drainage Diagram for Groton-Proposed O&M 2011-10-29
Prepared by Vanasse Hangen Brustlin, Inc., Printed 11/1/2011

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0.209	55	Woods, Good, HSG B (102)
0.891	61	>75% Grass cover, Good, HSG B (102, 203, 204, 205)
9.980	67	Based on CN for P101 (201, 202)
0.543	85	Gravel parking/roads, HSG B (203, 204, 205)
0.107	85	Gravel roads, HSG B (102)
0.092	98	Roof (201, 203)
11.822		**TOTAL AREA**

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment102: Flow to Brook Runoff Area=0.768 ac 0.00% Impervious Runoff Depth=0.05"
Flow Length=206' Tc=18.0 min CN=63 Runoff=0.00 cfs 0.003 af

Subcatchment201: To Stone Trench Drain Runoff Area=7.511 ac 0.61% Impervious Runoff Depth=0.12"
Flow Length=1,500' Tc=27.8 min CN=67 Runoff=0.13 cfs 0.074 af

Subcatchment202: To Culvert 101 Runoff Area=2.515 ac 0.00% Impervious Runoff Depth=0.12"
Flow Length=635' Tc=22.5 min CN=67 Runoff=0.04 cfs 0.025 af

Subcatchment203: O&M Site Runoff Area=0.546 ac 8.42% Impervious Runoff Depth=0.59"
Tc=6.0 min CN=80 Runoff=0.54 cfs 0.027 af

Subcatchment204: Overland to Filtration Runoff Area=0.377 ac 0.00% Impervious Runoff Depth=0.26"
Tc=6.0 min CN=72 Runoff=0.09 cfs 0.008 af

Subcatchment205: To Culvert Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=0.12"
Tc=6.0 min CN=67 Runoff=0.00 cfs 0.001 af

Pond 1P: Culvert 101 Peak Elev=682.09' Inflow=0.04 cfs 0.025 af
18.0" Round Culvert n=0.013 L=115.0' S=0.0200 '/' Outflow=0.04 cfs 0.025 af

Pond 104P: Filtration Basin Peak Elev=666.20' Storage=470 cf Inflow=0.62 cfs 0.109 af
Outflow=0.14 cfs 0.109 af

Pond HW103: Headwall Peak Elev=670.37' Inflow=0.54 cfs 0.101 af
12.0" Round Culvert n=0.013 L=40.0' S=0.0500 '/' Outflow=0.54 cfs 0.101 af

Pond HW105: Headwall Peak Elev=659.53' Inflow=0.00 cfs 0.001 af
12.0" Round Culvert n=0.013 L=82.0' S=0.0085 '/' Outflow=0.00 cfs 0.001 af

Pond MH104: Manhole Peak Elev=658.90' Inflow=0.14 cfs 0.110 af
12.0" Round Culvert n=0.013 L=29.0' S=0.0069 '/' Outflow=0.14 cfs 0.110 af

Pond ST101: Stone Trench Peak Elev=680.69' Inflow=0.13 cfs 0.074 af
Primary=0.13 cfs 0.074 af Secondary=0.00 cfs 0.000 af Outflow=0.13 cfs 0.074 af

Link 1: Flow to Brook Inflow=0.18 cfs 0.137 af
Primary=0.18 cfs 0.137 af

Total Runoff Area = 11.822 ac Runoff Volume = 0.137 af Average Runoff Depth = 0.14"
99.22% Pervious = 11.730 ac 0.78% Impervious = 0.092 ac

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment102: Flow to Brook Runoff Area=0.768 ac 0.00% Impervious Runoff Depth=0.10"
Flow Length=206' Tc=18.0 min CN=63 Runoff=0.01 cfs 0.007 af

Subcatchment201: To Stone Trench Drain Runoff Area=7.511 ac 0.61% Impervious Runoff Depth=0.21"
Flow Length=1,500' Tc=27.8 min CN=67 Runoff=0.38 cfs 0.130 af

Subcatchment202: To Culvert 101 Runoff Area=2.515 ac 0.00% Impervious Runoff Depth=0.21"
Flow Length=635' Tc=22.5 min CN=67 Runoff=0.13 cfs 0.044 af

Subcatchment203: O&M Site Runoff Area=0.546 ac 8.42% Impervious Runoff Depth=0.79"
Tc=6.0 min CN=80 Runoff=0.74 cfs 0.036 af

Subcatchment204: Overland to Filtration Runoff Area=0.377 ac 0.00% Impervious Runoff Depth=0.39"
Tc=6.0 min CN=72 Runoff=0.18 cfs 0.012 af

Subcatchment205: To Culvert Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=0.21"
Tc=6.0 min CN=67 Runoff=0.01 cfs 0.002 af

Pond 1P: Culvert 101 Peak Elev=682.16' Inflow=0.13 cfs 0.044 af
18.0" Round Culvert n=0.013 L=115.0' S=0.0200 '/' Outflow=0.13 cfs 0.044 af

Pond 104P: Filtration Basin Peak Elev=666.82' Storage=2,056 cf Inflow=0.92 cfs 0.178 af
Outflow=0.16 cfs 0.178 af

Pond HW103: Headwall Peak Elev=670.44' Inflow=0.74 cfs 0.166 af
12.0" Round Culvert n=0.013 L=40.0' S=0.0500 '/' Outflow=0.74 cfs 0.166 af

Pond HW105: Headwall Peak Elev=659.56' Inflow=0.01 cfs 0.002 af
12.0" Round Culvert n=0.013 L=82.0' S=0.0085 '/' Outflow=0.01 cfs 0.002 af

Pond MH104: Manhole Peak Elev=658.92' Inflow=0.16 cfs 0.180 af
12.0" Round Culvert n=0.013 L=29.0' S=0.0069 '/' Outflow=0.16 cfs 0.180 af

Pond ST101: Stone Trench Peak Elev=680.85' Inflow=0.38 cfs 0.130 af
Primary=0.38 cfs 0.130 af Secondary=0.00 cfs 0.000 af Outflow=0.38 cfs 0.130 af

Link 1: Flow to Brook Inflow=0.28 cfs 0.231 af
Primary=0.28 cfs 0.231 af

Total Runoff Area = 11.822 ac Runoff Volume = 0.230 af Average Runoff Depth = 0.23"
99.22% Pervious = 11.730 ac 0.78% Impervious = 0.092 ac

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment102: Flow to Brook Runoff Area=0.768 ac 0.00% Impervious Runoff Depth=0.67"
Flow Length=206' Tc=18.0 min CN=63 Runoff=0.38 cfs 0.043 af

Subcatchment201: To Stone Trench Drain Runoff Area=7.511 ac 0.61% Impervious Runoff Depth=0.91"
Flow Length=1,500' Tc=27.8 min CN=67 Runoff=4.70 cfs 0.570 af

Subcatchment202: To Culvert 101 Runoff Area=2.515 ac 0.00% Impervious Runoff Depth=0.91"
Flow Length=635' Tc=22.5 min CN=67 Runoff=1.83 cfs 0.191 af

Subcatchment203: O&M Site Runoff Area=0.546 ac 8.42% Impervious Runoff Depth=1.92"
Tc=6.0 min CN=80 Runoff=1.85 cfs 0.087 af

Subcatchment204: Overland to Filtration Runoff Area=0.377 ac 0.00% Impervious Runoff Depth=1.26"
Tc=6.0 min CN=72 Runoff=0.82 cfs 0.040 af

Subcatchment205: To Culvert Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=0.91"
Tc=6.0 min CN=67 Runoff=0.15 cfs 0.008 af

Pond 1P: Culvert 101 Peak Elev=682.92' Inflow=3.73 cfs 0.236 af
18.0" Round Culvert n=0.013 L=115.0' S=0.0200 '/' Outflow=3.73 cfs 0.236 af

Pond 104P: Filtration Basin Peak Elev=668.13' Storage=6,342 cf Inflow=3.53 cfs 0.652 af
Outflow=2.75 cfs 0.610 af

Pond HW103: Headwall Peak Elev=671.24' Inflow=3.25 cfs 0.612 af
12.0" Round Culvert n=0.013 L=40.0' S=0.0500 '/' Outflow=3.25 cfs 0.612 af

Pond HW105: Headwall Peak Elev=659.87' Inflow=0.15 cfs 0.008 af
12.0" Round Culvert n=0.013 L=82.0' S=0.0085 '/' Outflow=0.15 cfs 0.008 af

Pond MH104: Manhole Peak Elev=659.87' Inflow=2.77 cfs 0.618 af
12.0" Round Culvert n=0.013 L=29.0' S=0.0069 '/' Outflow=2.77 cfs 0.618 af

Pond ST101: Stone Trench Peak Elev=684.42' Inflow=4.70 cfs 0.570 af
Primary=2.68 cfs 0.525 af Secondary=2.02 cfs 0.045 af Outflow=4.70 cfs 0.570 af

Link 1: Flow to Brook Inflow=4.26 cfs 0.896 af
Primary=4.26 cfs 0.896 af

Total Runoff Area = 11.822 ac Runoff Volume = 0.939 af Average Runoff Depth = 0.95"
99.22% Pervious = 11.730 ac 0.78% Impervious = 0.092 ac

Time span=5.00-30.00 hrs, dt=0.05 hrs, 501 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment102: Flow to Brook Runoff Area=0.768 ac 0.00% Impervious Runoff Depth=1.33"
Flow Length=206' Tc=18.0 min CN=63 Runoff=1.01 cfs 0.085 af

Subcatchment201: To Stone Trench Drain Runoff Area=7.511 ac 0.61% Impervious Runoff Depth=1.67"
Flow Length=1,500' Tc=27.8 min CN=67 Runoff=10.30 cfs 1.045 af

Subcatchment202: To Culvert 101 Runoff Area=2.515 ac 0.00% Impervious Runoff Depth=1.67"
Flow Length=635' Tc=22.5 min CN=67 Runoff=3.97 cfs 0.350 af

Subcatchment203: O&M Site Runoff Area=0.546 ac 8.42% Impervious Runoff Depth=2.94"
Tc=6.0 min CN=80 Runoff=2.81 cfs 0.134 af

Subcatchment204: Overland to Filtration Runoff Area=0.377 ac 0.00% Impervious Runoff Depth=2.13"
Tc=6.0 min CN=72 Runoff=1.43 cfs 0.067 af

Subcatchment205: To Culvert Runoff Area=0.105 ac 0.00% Impervious Runoff Depth=1.67"
Tc=6.0 min CN=67 Runoff=0.30 cfs 0.015 af

Pond 1P: Culvert 101 Peak Elev=684.52' Inflow=11.32 cfs 0.612 af
18.0" Round Culvert n=0.013 L=115.0' S=0.0200 '/' Outflow=11.32 cfs 0.612 af

Pond 104P: Filtration Basin Peak Elev=668.16' Storage=6,440 cf Inflow=6.75 cfs 0.984 af
Outflow=3.19 cfs 0.940 af

Pond HW103: Headwall Peak Elev=672.50' Inflow=5.35 cfs 0.917 af
12.0" Round Culvert n=0.013 L=40.0' S=0.0500 '/' Outflow=5.35 cfs 0.917 af

Pond HW105: Headwall Peak Elev=660.14' Inflow=0.30 cfs 0.015 af
12.0" Round Culvert n=0.013 L=82.0' S=0.0085 '/' Outflow=0.30 cfs 0.015 af

Pond MH104: Manhole Peak Elev=660.13' Inflow=3.23 cfs 0.954 af
12.0" Round Culvert n=0.013 L=29.0' S=0.0069 '/' Outflow=3.23 cfs 0.954 af

Pond ST101: Stone Trench Peak Elev=684.77' Inflow=10.30 cfs 1.045 af
Primary=2.73 cfs 0.783 af Secondary=7.57 cfs 0.262 af Outflow=10.30 cfs 1.045 af

Link 1: Flow to Brook Inflow=14.80 cfs 1.652 af
Primary=14.80 cfs 1.652 af

Total Runoff Area = 11.822 ac Runoff Volume = 1.696 af Average Runoff Depth = 1.72"
99.22% Pervious = 11.730 ac 0.78% Impervious = 0.092 ac

Summary for Subcatchment 102: Flow to Brook

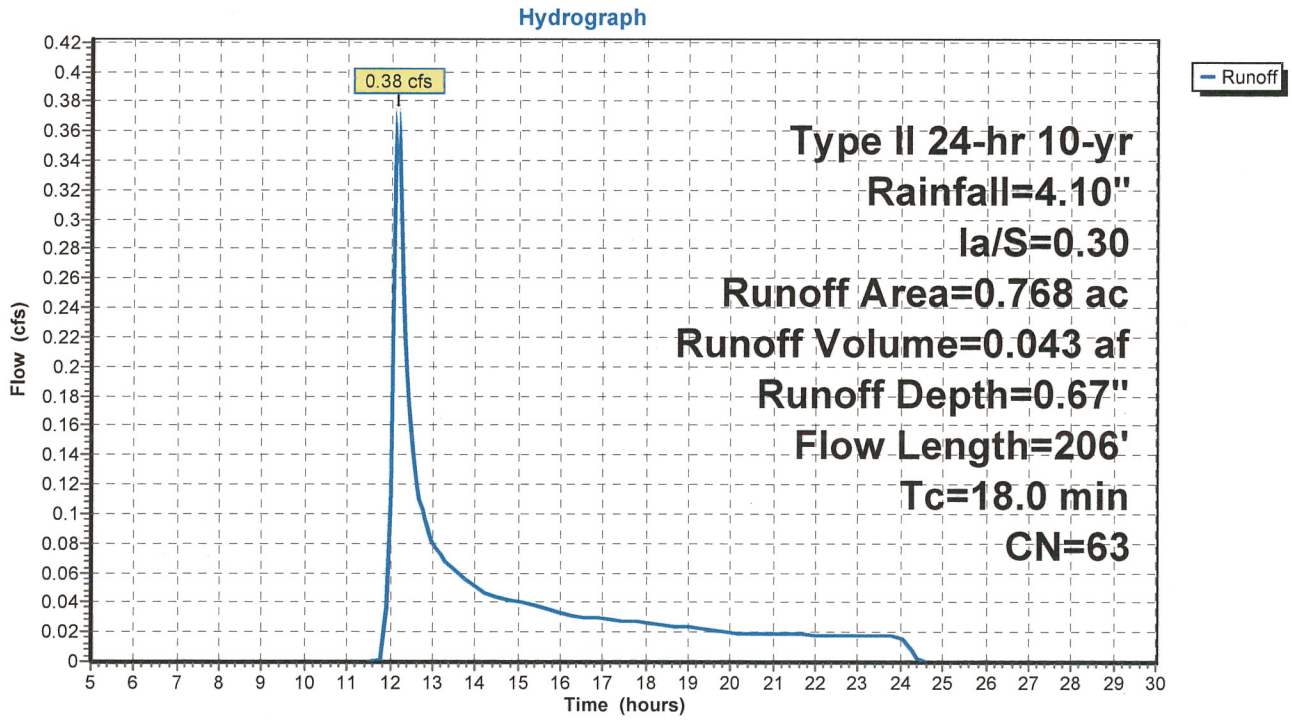
Runoff = 0.38 cfs @ 12.16 hrs, Volume= 0.043 af, Depth= 0.67"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

Area (ac)	CN	Description
0.107	85	Gravel roads, HSG B
0.452	61	>75% Grass cover, Good, HSG B
0.209	55	Woods, Good, HSG B
0.768	63	Weighted Average
0.768		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.2	100	0.1800	0.10		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.60"
0.8	106	0.2000	2.24		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
18.0	206	Total			

Subcatchment 102: Flow to Brook



Summary for Subcatchment 201: To Stone Trench Drain

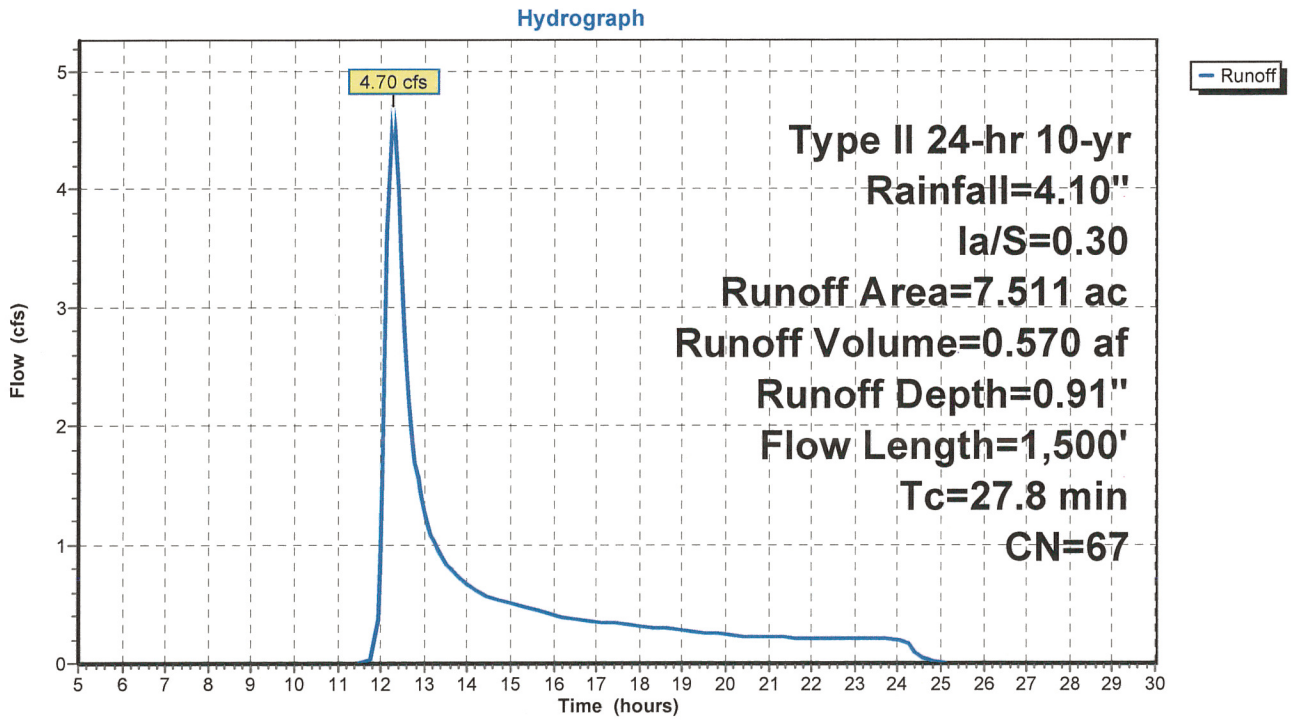
Runoff = 4.70 cfs @ 12.27 hrs, Volume= 0.570 af, Depth= 0.91"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

	Area (ac)	CN	Description
*	7.465	67	Based on CN for P101
*	0.046	98	Roof
	7.511	67	Weighted Average
	7.465		99.39% Pervious Area
	0.046		0.61% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
18.5	100	0.1500	0.09		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.60"
9.3	1,400	0.2500	2.50		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
27.8	1,500	Total			

Subcatchment 201: To Stone Trench Drain



Summary for Subcatchment 202: To Culvert 101

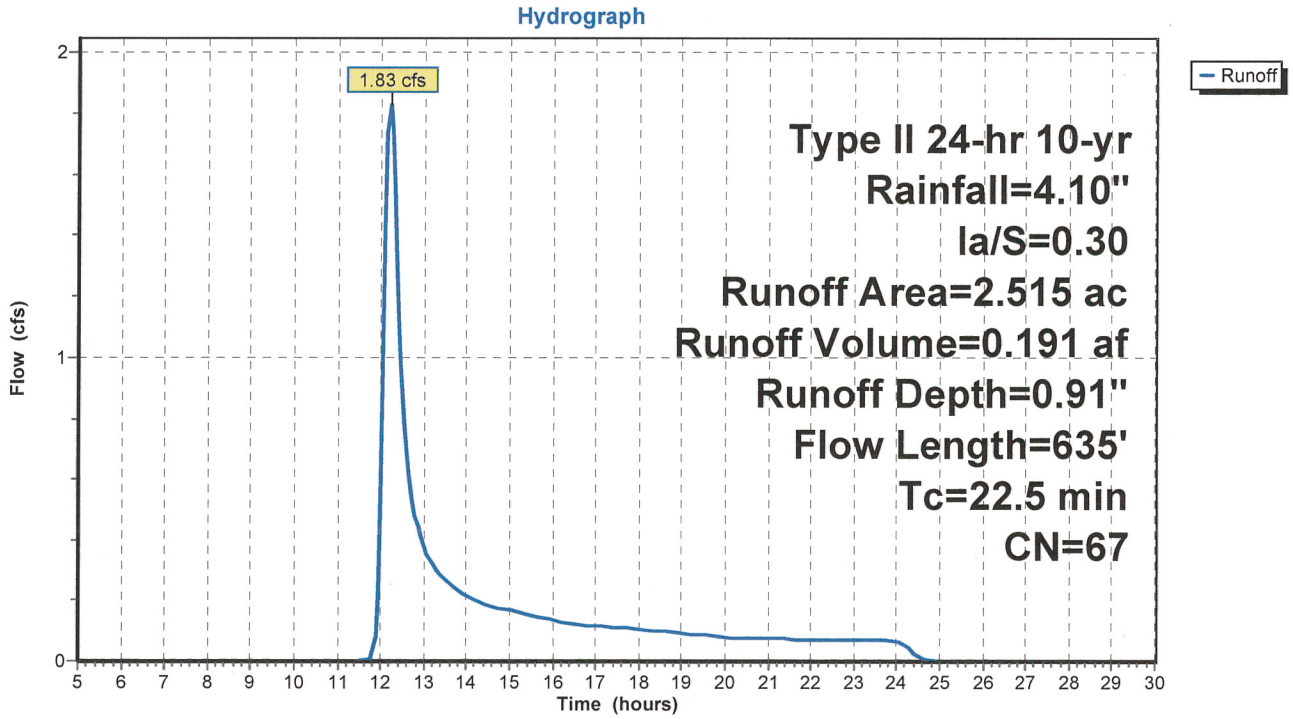
Runoff = 1.83 cfs @ 12.20 hrs, Volume= 0.191 af, Depth= 0.91"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

	Area (ac)	CN	Description
*	2.515	67	Based on CN for P101
	2.515		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
17.6	100	0.1700	0.09		**Sheet Flow,** Woods: Dense underbrush n= 0.800 P2= 2.60"
4.9	535	0.1340	1.83		**Shallow Concentrated Flow,** Woodland Kv= 5.0 fps
22.5	635	Total			

Subcatchment 202: To Culvert 101



Summary for Subcatchment 203: O&M Site

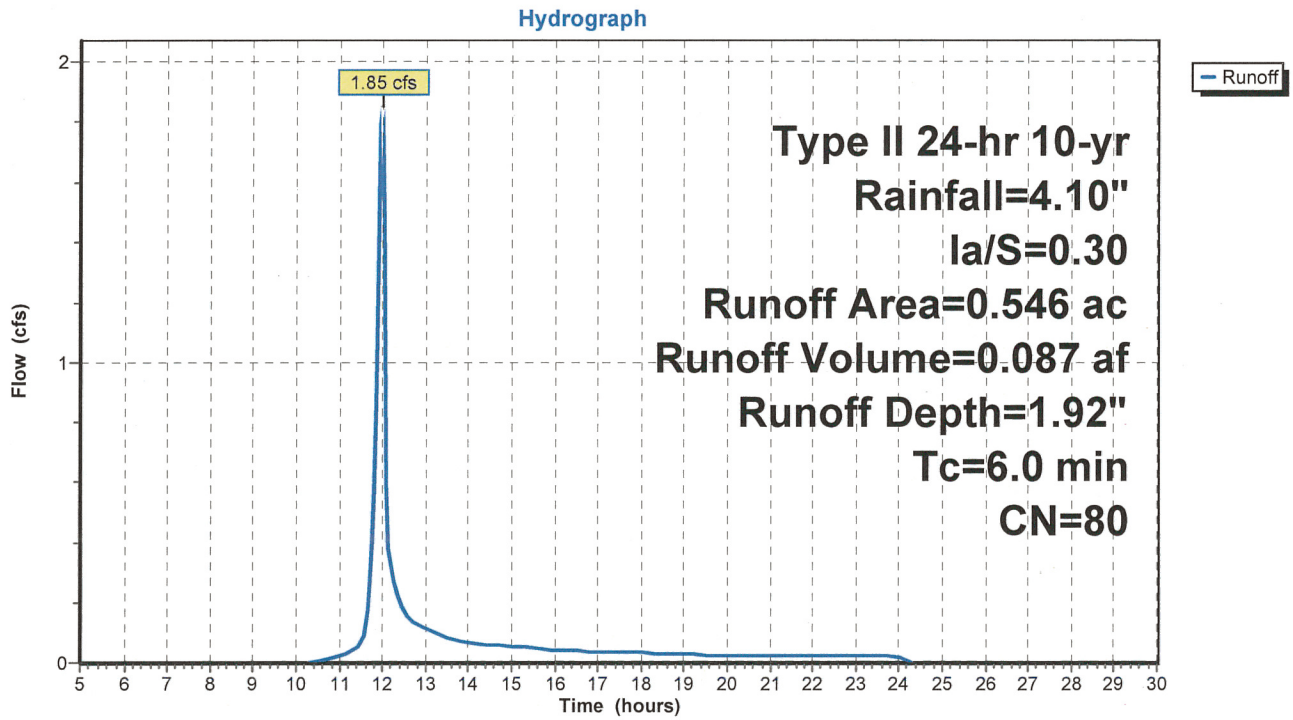
Runoff = 1.85 cfs @ 11.97 hrs, Volume= 0.087 af, Depth= 1.92"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

	Area (ac)	CN	Description
	0.150	61	>75% Grass cover, Good, HSG B
*	0.350	85	Gravel parking/roads, HSG B
*	0.046	98	Roof
	0.546	80	Weighted Average
	0.500		91.58% Pervious Area
	0.046		8.42% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry,**

Subcatchment 203: O&M Site



Summary for Subcatchment 204: Overland to Filtration Basin

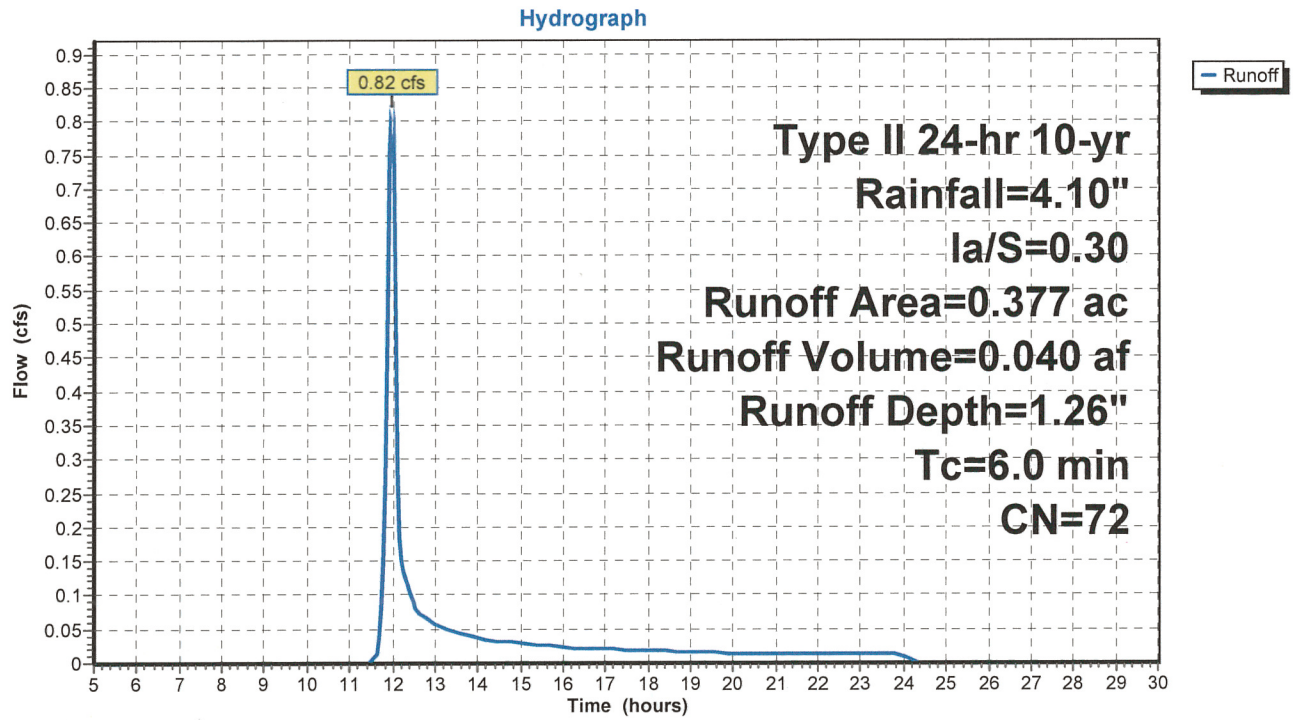
Runoff = 0.82 cfs @ 11.98 hrs, Volume= 0.040 af, Depth= 1.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

	Area (ac)	CN	Description
	0.210	61	>75% Grass cover, Good, HSG B
*	0.167	85	Gravel parking/roads, HSG B
	0.377	72	Weighted Average
	0.377		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry,**

Subcatchment 204: Overland to Filtration Basin



Summary for Subcatchment 205: To Culvert

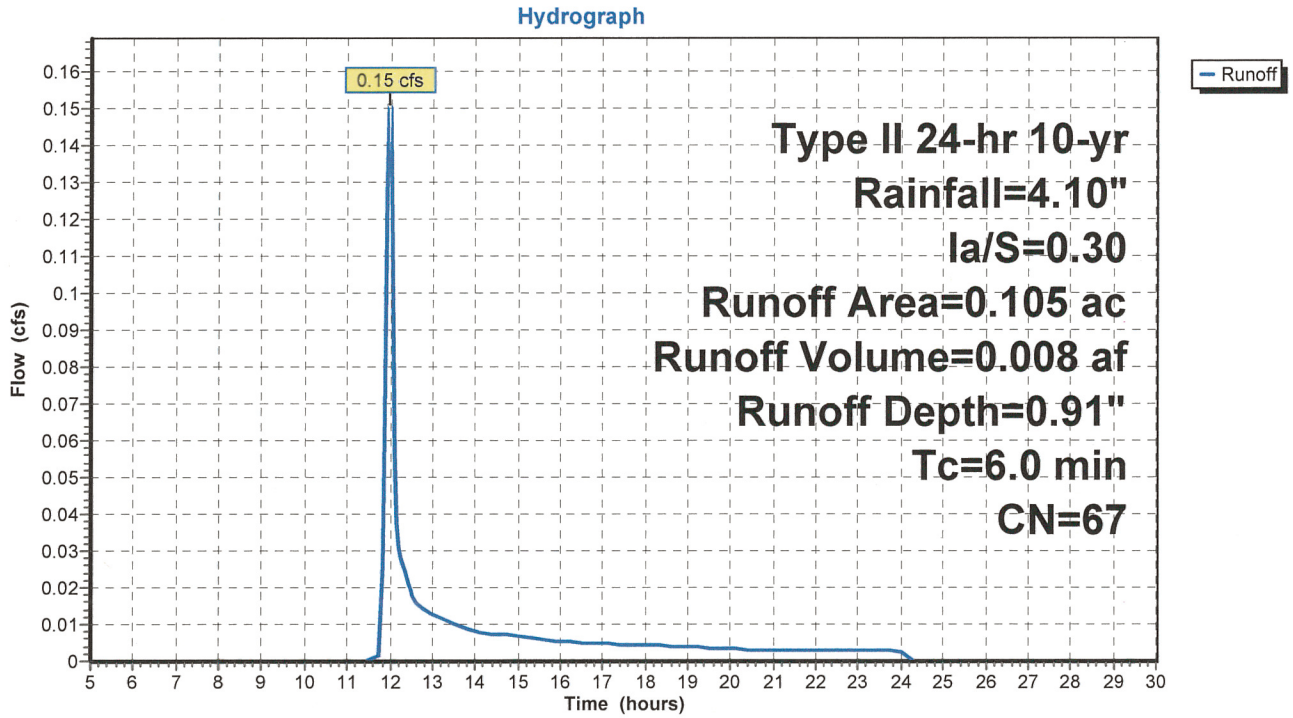
Runoff = 0.15 cfs @ 11.99 hrs, Volume= 0.008 af, Depth= 0.91"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Type II 24-hr 10-yr Rainfall=4.10", Ia/S=0.30

	Area (ac)	CN	Description
	0.079	61	>75% Grass cover, Good, HSG B
*	0.026	85	Gravel parking/roads, HSG B
	0.105	67	Weighted Average
	0.105		100.00% Pervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0					**Direct Entry,**

Subcatchment 205: To Culvert



Summary for Pond 1P: Culvert 101

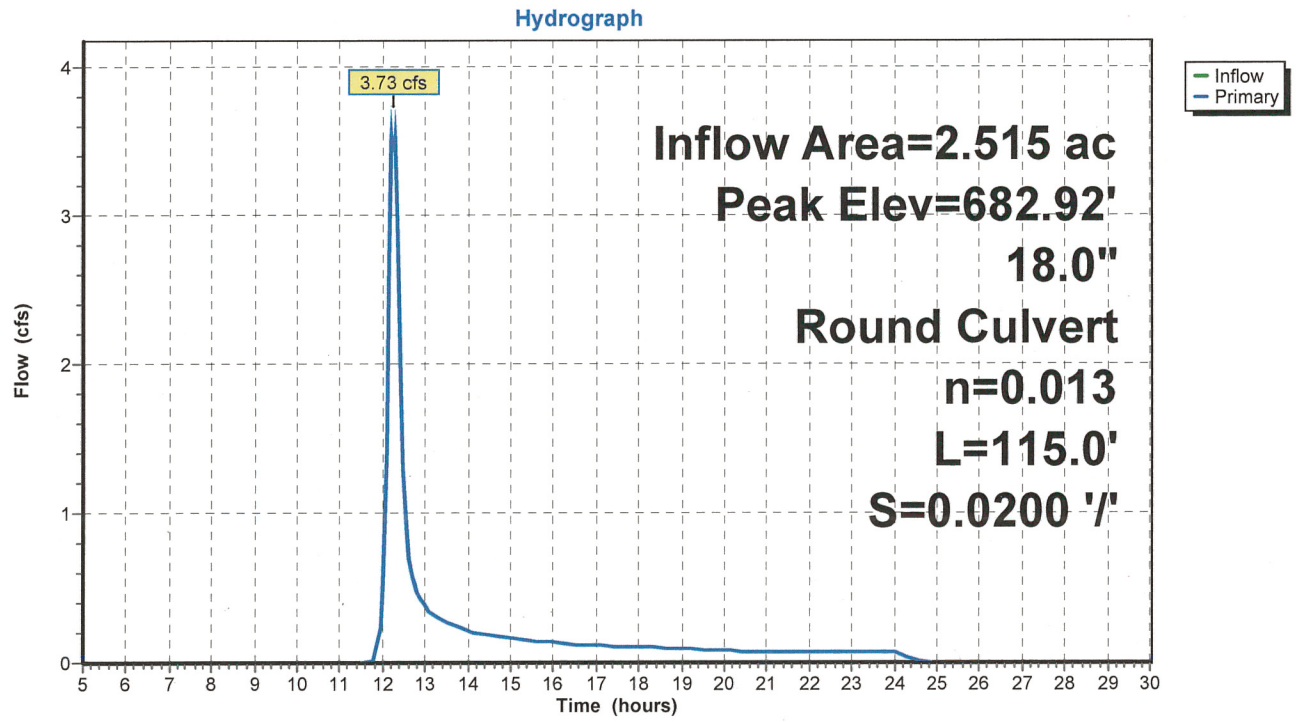
Inflow Area = 2.515 ac, 0.00% Impervious, Inflow Depth = 1.13" for 10-yr event
Inflow = 3.73 cfs @ 12.25 hrs, Volume= 0.236 af
Outflow = 3.73 cfs @ 12.25 hrs, Volume= 0.236 af, Atten= 0%, Lag= 0.0 min
Primary = 3.73 cfs @ 12.25 hrs, Volume= 0.236 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Peak Elev= 682.92' @ 12.25 hrs
Flood Elev= 685.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	682.00'	**18.0" Round Culvert** L= 115.0' CPP, square edge headwall, Ke= 0.500 Outlet Invert= 679.70' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior

Primary OutFlow Max=3.72 cfs @ 12.25 hrs HW=682.92' TW=0.00' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.72 cfs @ 3.27 fps)

Pond 1P: Culvert 101



Summary for Pond 104P: Filtration Basin

Inflow Area = 8.434 ac, 1.09% Impervious, Inflow Depth = 0.93" for 10-yr event
Inflow = 3.53 cfs @ 12.09 hrs, Volume= 0.652 af
Outflow = 2.75 cfs @ 12.60 hrs, Volume= 0.610 af, Atten= 22%, Lag= 30.1 min
Primary = 2.75 cfs @ 12.60 hrs, Volume= 0.610 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Peak Elev= 668.13' @ 12.59 hrs Surf.Area= 3,734 sf Storage= 6,342 cf
Flood Elev= 669.20' Surf.Area= 4,690 sf Storage= 10,796 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)
Center-of-Mass det. time= 141.9 min (1,055.8 - 913.9)

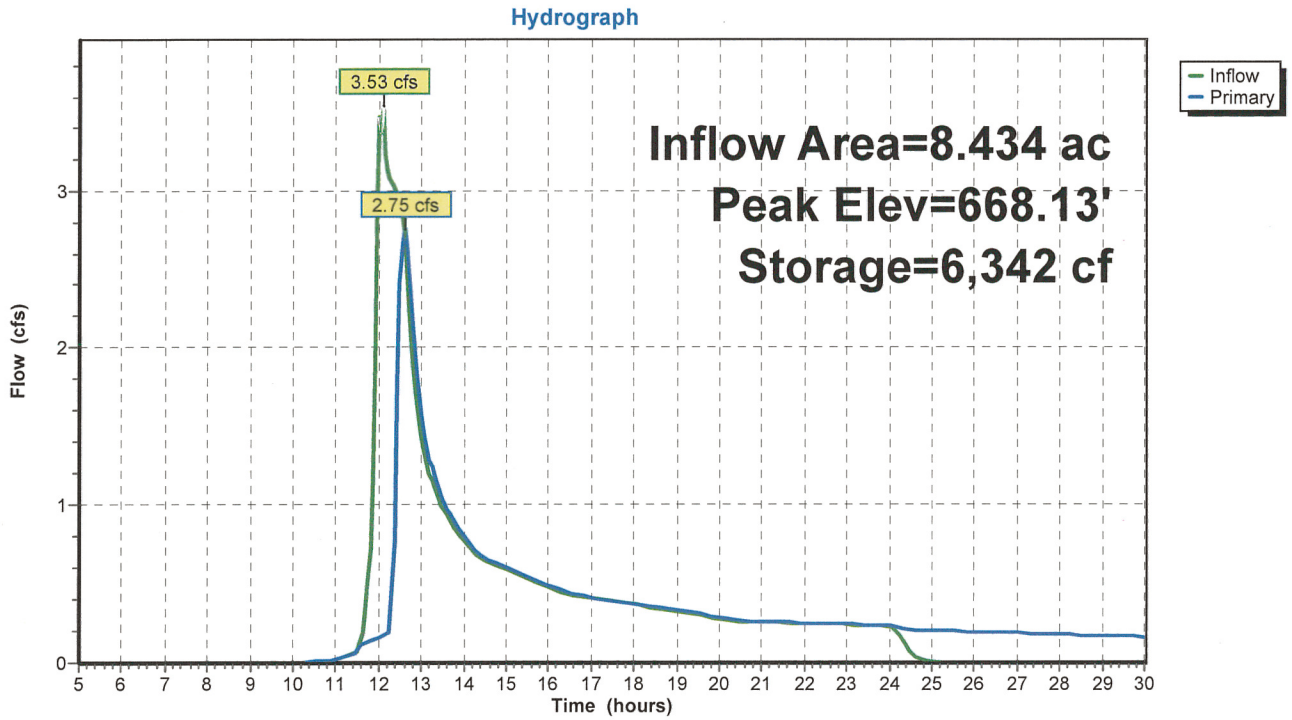
Volume	Invert	Avail.Storage	Storage Description
#1	666.00'	10,796 cf	**Custom Stage Data (Prismatic)**Listed below (Recalc)

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
666.00	2,230	0	0
668.00	3,631	5,861	5,861
669.00	4,417	4,024	9,885
669.20	4,690	911	10,796

Device	Routing	Invert	Outlet Devices
#1	Primary	662.80'	**12.0" Round Culvert** L= 24.0' CPP, square edge headwall, Ke= 0.500 Outlet Invert= 662.00' S= 0.0333 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior
#2	Device 1	666.00'	**2.500 in/hr Exfiltration to UD's over Surface area**
#3	Device 1	667.90'	**2.3" x 4.6" Horiz. Orifice/Grate X 4.00 columns** X 8 rows C= 0.600 in 21.0" x 21.0" Grate Limited to weir flow at low heads

Primary OutFlow Max=2.74 cfs @ 12.60 hrs HW=668.13' TW=659.87' (Dynamic Tailwater)
└─**1=Culvert** (Passes 2.74 cfs of 8.31 cfs potential flow)
  ├─**2=Exfiltration to UD's** (Exfiltration Controls 0.22 cfs)
  └─**3=Orifice/Grate** (Weir Controls 2.53 cfs @ 1.57 fps)

Pond 104P: Filtration Basin



Summary for Pond HW103: Headwall

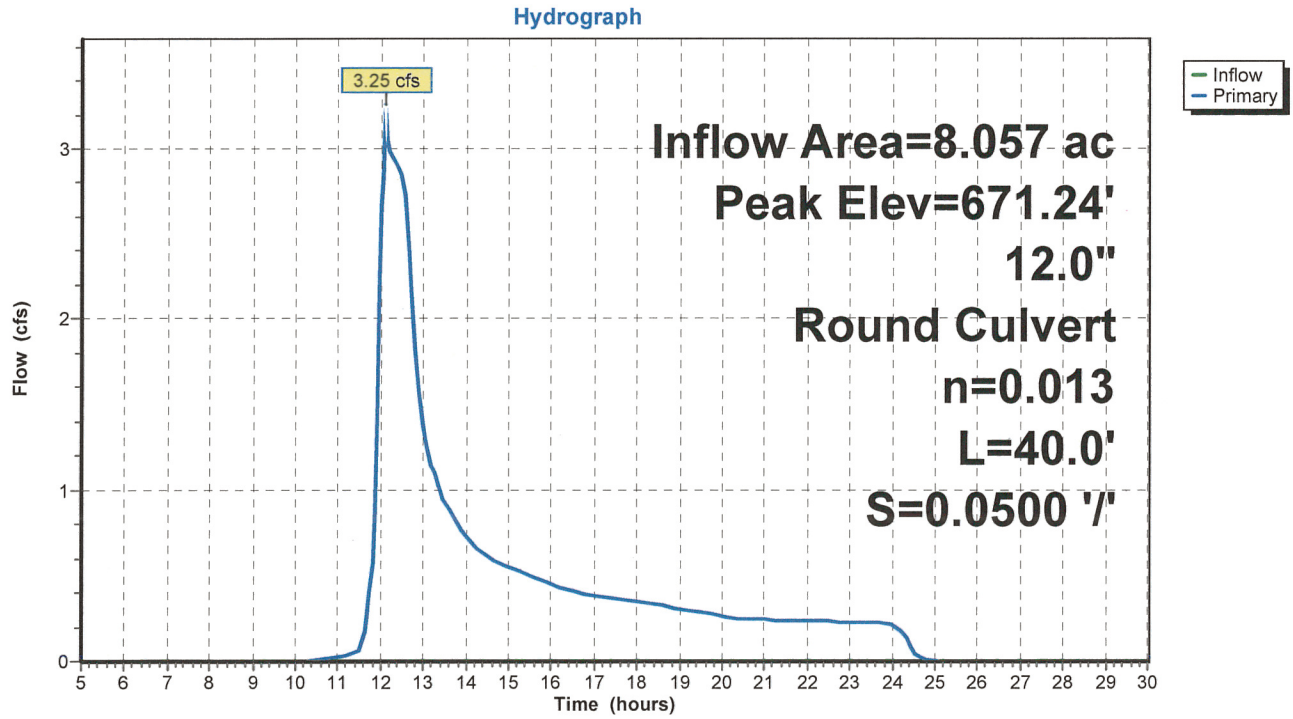
Inflow Area = 8.057 ac, 1.14% Impervious, Inflow Depth = 0.91" for 10-yr event
Inflow = 3.25 cfs @ 12.11 hrs, Volume= 0.612 af
Outflow = 3.25 cfs @ 12.11 hrs, Volume= 0.612 af, Atten= 0%, Lag= 0.0 min
Primary = 3.25 cfs @ 12.11 hrs, Volume= 0.612 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Peak Elev= 671.24' @ 12.11 hrs
Flood Elev= 673.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	670.00'	**12.0" Round Culvert** L= 40.0' CPP, square edge headwall, Ke= 0.500 Outlet Invert= 668.00' S= 0.0500 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior

Primary OutFlow Max=3.21 cfs @ 12.11 hrs HW=671.22' TW=667.07' (Dynamic Tailwater)
1=Culvert (Inlet Controls 3.21 cfs @ 4.08 fps)

Pond HW103: Headwall



Summary for Pond HW105: Headwall

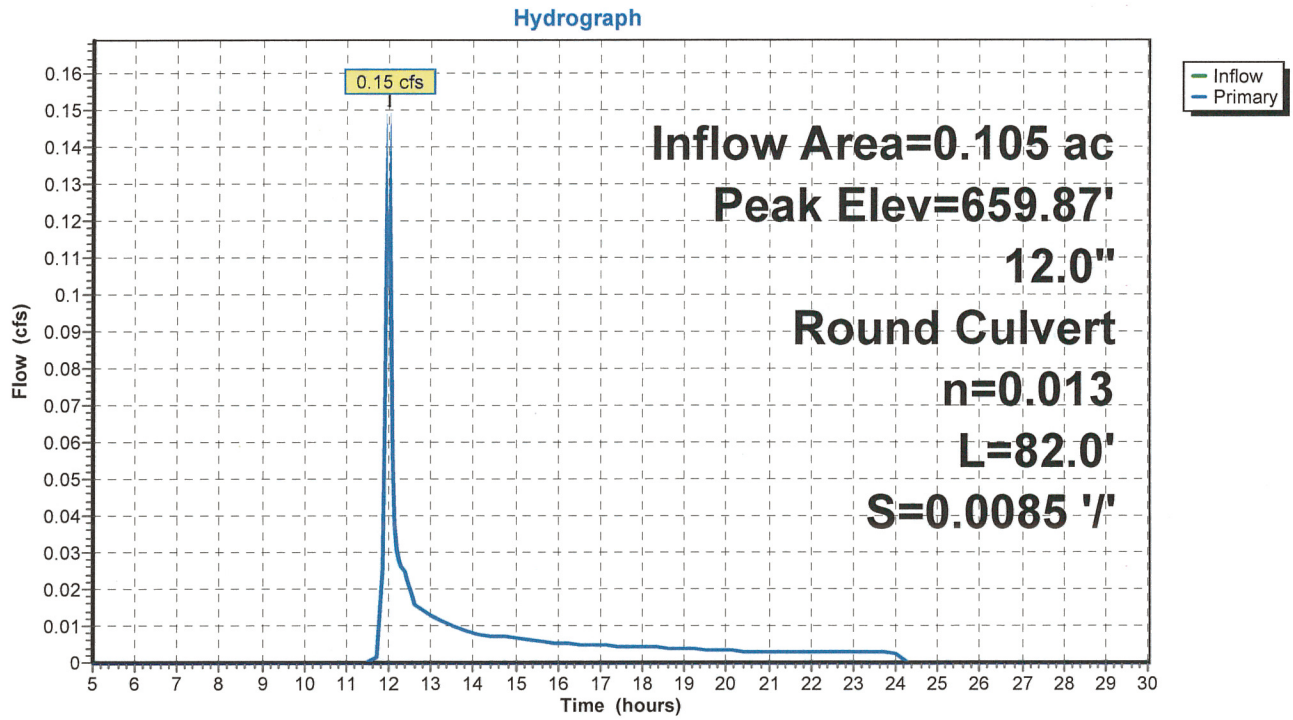
Inflow Area = 0.105 ac, 0.00% Impervious, Inflow Depth = 0.91" for 10-yr event
Inflow = 0.15 cfs @ 11.99 hrs, Volume= 0.008 af
Outflow = 0.15 cfs @ 11.99 hrs, Volume= 0.008 af, Atten= 0%, Lag= 0.0 min
Primary = 0.15 cfs @ 11.99 hrs, Volume= 0.008 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Peak Elev= 659.87' @ 12.64 hrs
Flood Elev= 662.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	659.50'	**12.0" Round Culvert** L= 82.0' CPP, square edge headwall, Ke= 0.500 Outlet Invert= 658.80' S= 0.0085 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior

Primary OutFlow Max=0.14 cfs @ 11.99 hrs HW=659.70' TW=659.00' (Dynamic Tailwater)
1=Culvert (Outlet Controls 0.14 cfs @ 1.99 fps)

Pond HW105: Headwall



Summary for Pond MH104: Manhole

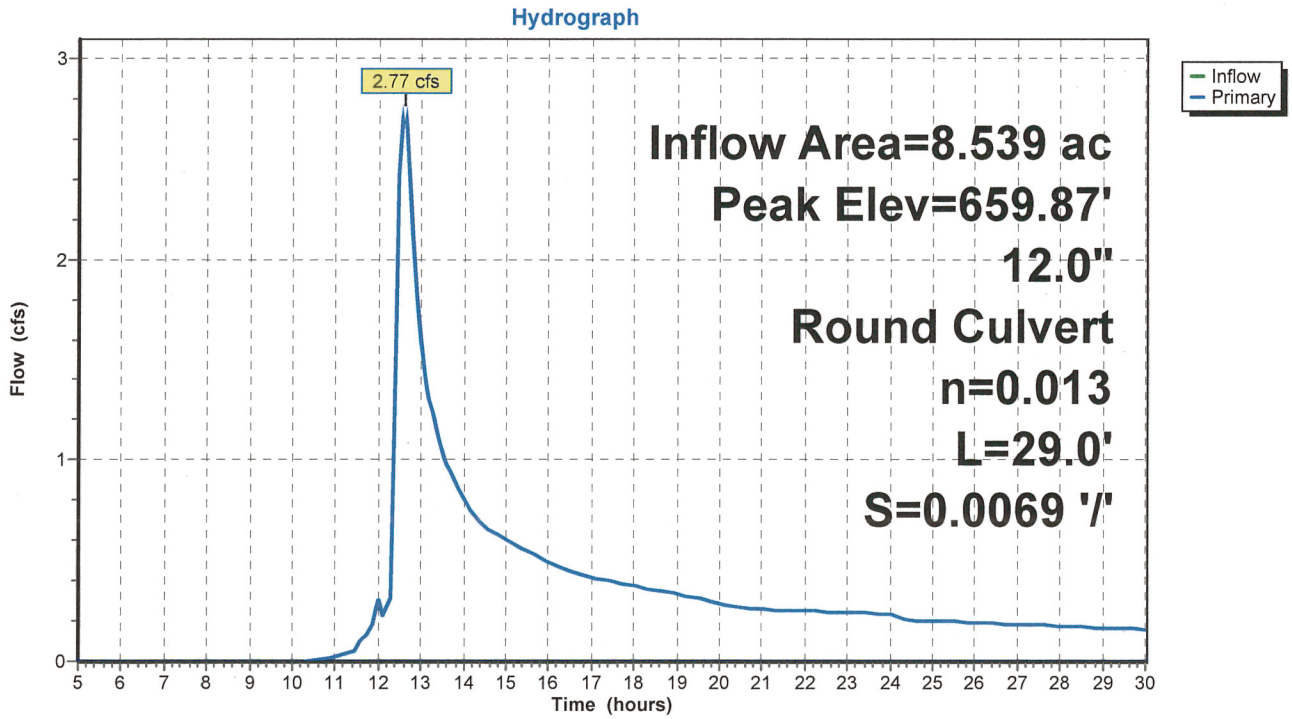
Inflow Area = 8.539 ac, 1.08% Impervious, Inflow Depth > 0.87" for 10-yr event
Inflow = 2.77 cfs @ 12.59 hrs, Volume= 0.618 af
Outflow = 2.77 cfs @ 12.59 hrs, Volume= 0.618 af, Atten= 0%, Lag= 0.0 min
Primary = 2.77 cfs @ 12.59 hrs, Volume= 0.618 af

Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Peak Elev= 659.87' @ 12.59 hrs
Flood Elev= 665.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	658.70'	**12.0" Round Culvert** L= 29.0' CPP, square edge headwall, Ke= 0.500 Outlet Invert= 658.50' S= 0.0069 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior

Primary OutFlow Max=2.76 cfs @ 12.59 hrs HW=659.87' TW=0.00' (Dynamic Tailwater)
1=Culvert (Barrel Controls 2.76 cfs @ 3.79 fps)

Pond MH104: Manhole



Summary for Pond ST101: Stone Trench

Inflow Area = 7.511 ac, 0.61% Impervious, Inflow Depth = 0.91" for 10-yr event
Inflow = 4.70 cfs @ 12.27 hrs, Volume= 0.570 af
Outflow = 4.70 cfs @ 12.27 hrs, Volume= 0.570 af, Atten= 0%, Lag= 0.0 min
Primary = 2.68 cfs @ 12.27 hrs, Volume= 0.525 af
Secondary = 2.02 cfs @ 12.27 hrs, Volume= 0.045 af

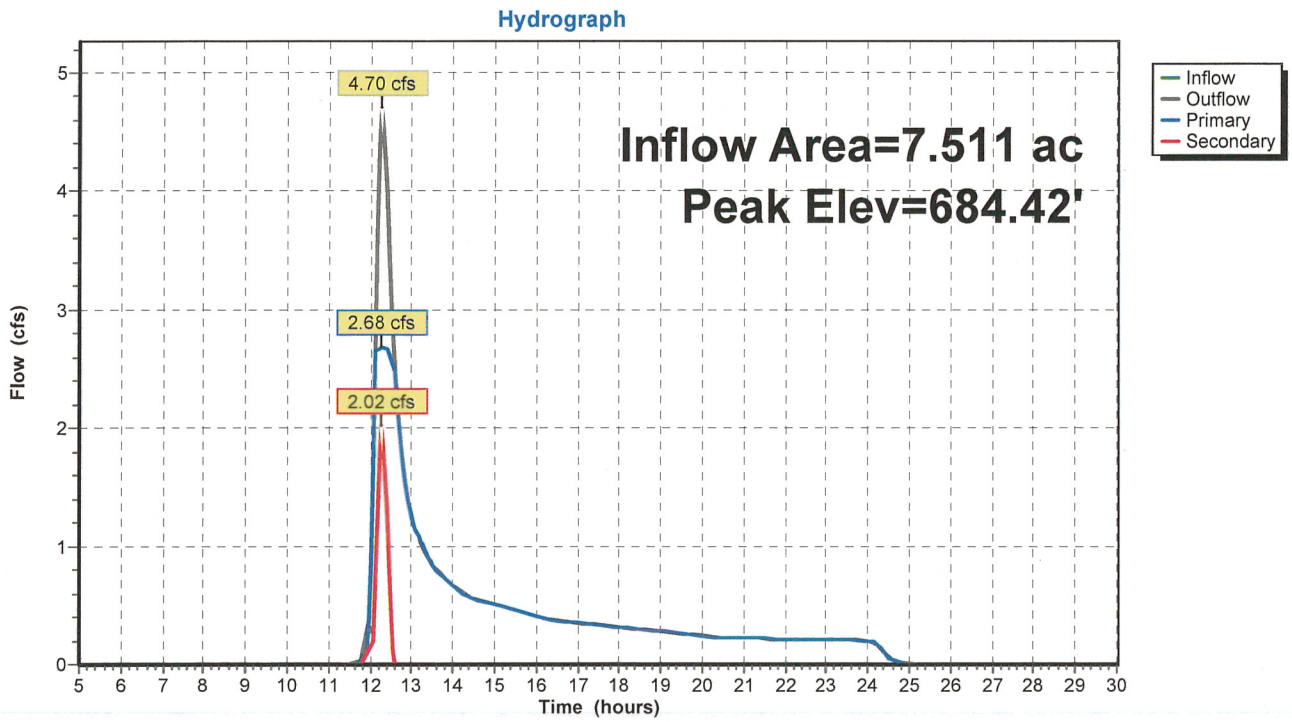
Routing by Dyn-Stor-Ind method, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs
Peak Elev= 684.42' @ 12.27 hrs
Flood Elev= 685.00'

Device	Routing	Invert	Outlet Devices
#1	Primary	680.50'	**8.0" Round Culvert** L= 170.0' CPP, square edge headwall, Ke= 0.500 Outlet Invert= 674.00' S= 0.0382 '/' Cc= 0.900 n= 0.013
#2	Secondary	684.20'	**8.0' long x 4.0' breadth Broad-Crested Rectangular Weir** Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.38 2.54 2.69 2.68 2.67 2.67 2.65 2.66 2.66 2.68 2.72 2.73 2.76 2.79 2.88 3.07 3.32

Primary OutFlow Max=2.68 cfs @ 12.27 hrs HW=684.42' TW=671.10' (Dynamic Tailwater)
└**1=Culvert** (Barrel Controls 2.68 cfs @ 7.67 fps)

Secondary OutFlow Max=1.98 cfs @ 12.27 hrs HW=684.42' TW=682.91' (Dynamic Tailwater)
└**2=Broad-Crested Rectangular Weir** (Weir Controls 1.98 cfs @ 1.12 fps)

Pond ST101: Stone Trench

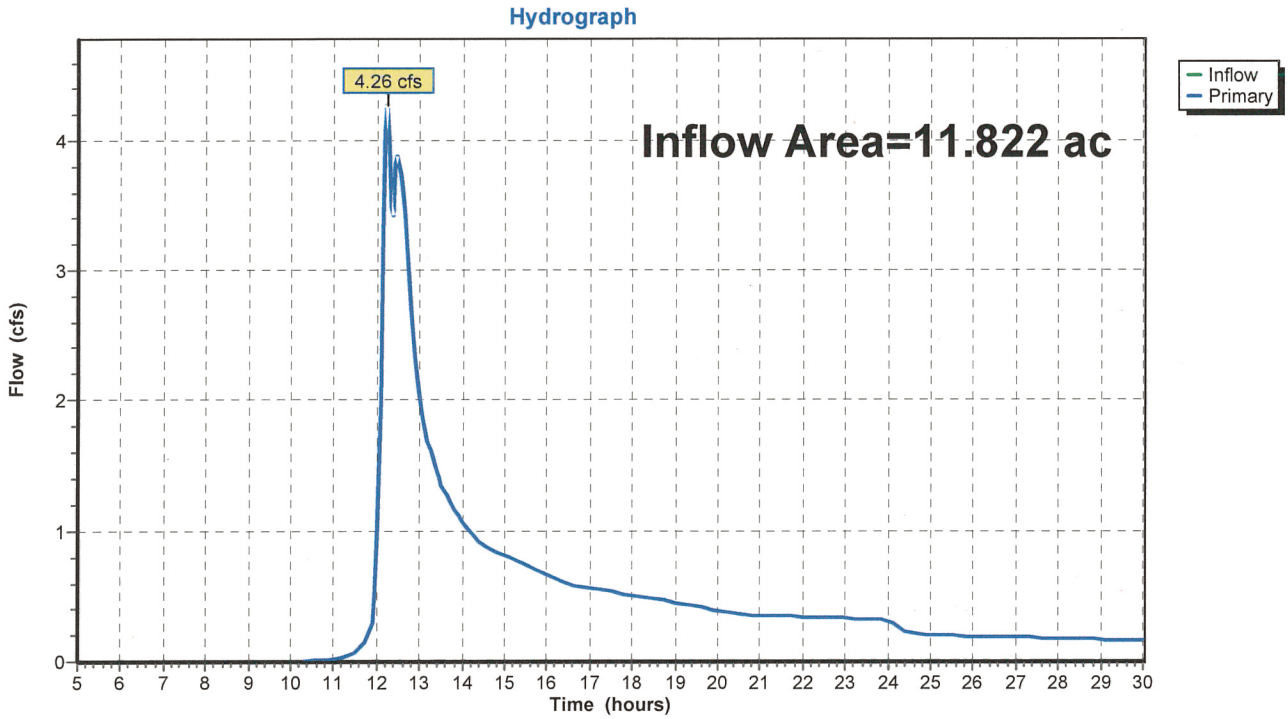


Summary for Link 1: Flow to Brook

Inflow Area = 11.822 ac, 0.78% Impervious, Inflow Depth > 0.91" for 10-yr event
Inflow = 4.26 cfs @ 12.25 hrs, Volume= 0.896 af
Primary = 4.26 cfs @ 12.25 hrs, Volume= 0.896 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-30.00 hrs, dt= 0.05 hrs

Link 1: Flow to Brook



General Calculations - WQV and WQF
(optional worksheet)

This worksheet may be useful when designing a BMP that does not fit into one of the specific worksheets already provided. For example, if proposing a new technology, which is not a stormwater wetland, infiltration practice, etc., then this worksheet may be useful.

Water Quality Volume (WQV)

8.44	ac	A = Area draining to the practice
0.62	ac	A_I = Impervious area draining to the practice
0.07	decimal	I = percent impervious area draining to the practice, in decimal form
0.12	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)
0.98	ac-in	WQV= 1" x Rv x A
3,563	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")

Water Quality Flow (WQF)

1	inches	P = amount of rainfall. For WQF in NH, P = 1".
0.12	inches	Q = water quality depth. Q = WQV/A
82	unitless	CN = unit peak discharge curve number. $CN = 1000/(10+5P+10Q-10*[Q^2 + 1.25*Q*P]^{0.5})$
2.2	inches	S = potential maximum retention. S = (1000/CN) - 10
0.435	inches	Ia = initial abstraction. Ia = 0.2S
6.0	minutes	T_c = Time of Concentration
280.0	$cfs/mi^2/in$	qu is the unit peak discharge. Obtain this value from TR-55 exhibits 4-II and 4-III
0.429	cfs	WQF = q_u x WQV. Conversion: to convert "$cfs/mi^2/in$ * ac-in" to "cfs" multiply by $1mi^2/640ac$

Designer's Notes:

For O&M Building Site Area

Filtration Basin has a cumulative storage of 10,796 cubic-feet

WQV in Basin set to 3,808 cubic-feet at elevation 667.4

Sediment forbay volume set to 25% of WQV or 952 cubic-feet

Impervious Area also includes gravel areas.

FILTRATION PRACTICE DESIGN CRITERIA (Env-Wq 1508.06)

Type/Node Name: **Proposed Filtration Basin**

Enter the type of filtration practice (e.g., bioretention system) and the node name in the drainage analysis, if applicable

YES		Have you reviewed the restrictions on unlined systems outlined in Env-Wq 1508.06(b)?	
8.44	ac	A = Area draining to the practice[1]	
0.62	ac	A_I = Impervious area draining to the practice	
0.07	decimal	I = percent impervious area draining to the practice, in decimal form	
0.12	unitless	Rv = Runoff coefficient = 0.05 + (0.9 x I)	
0.98	ac-in	WQV= 1" x Rv x A	
3,557	cf	WQV conversion (ac-in x 43,560 sf/ac x 1ft/12")	
889	cf	25% x WQV (check calc for sediment forebay volume)	
2,668	cf	75% x WQV (check calc for surface sand filter volume)	
Forebay		Method of Pretreatment? (not required for clean or roof runoff)	
952	cf	V_{SED} = sediment forebay volume, if used for pretreatment	← ≥ 25%WQV
2,230	sf	A_{SA} = surface area of the practice	
2.5	iph	I_{DESIGN} = design infiltration rate[2]	
YES	Yes/No	If I_{DESIGN} is < 0.50 iph, has an underdrain been provided?	
2.0	hours	T_{DRAIN} = drain time = V_{PP} / (A_{SA} * I_{DESIGN})	← ≤ 72-hrs
664.25	feet	E_{FC} = elevation of the bottom of the filter course material	
663.00	feet	E_{UD} = invert elevation of the underdrain (UD), if applicable	
662.50	feet	E_{BTM} = elevation of the bottom of the practice (i.e., bottom of the stone reservoir).	
NA	feet	E_{SHWT} = elevation of SHWT (if none found, enter the lowest elevation of the test pit)	
NA	feet	E_{ROCK} = elevation of bedrock (if none found, enter the lowest elevation of the test pit)	
1.25	feet	$D_{FC\ to\ UD}$ = depth to UD from the bottom of the filter course[3]	← ≥ 1'
#VALUE!	feet	$D_{FC\ to\ ROCK}$ = depth to bedrock from the bottom of the filter course[3]	← ≥ 1'
#VALUE!	feet	$D_{FC\ to\ SHWT}$ = depth to SHWT from the bottom of the filter course[3]	← ≥ 1'
#VALUE!	feet	$D_{BTM\ to\ SHWT}$ = depth to SHWT from the bottom of the practice[3]	← ≥ 2'
668.13	ft	Peak elevation of the 10-year storm event (infiltration can be used in analysis)	
669.20	ft	Elevation of the top of the practice	
YES		10 peak elevation ≤ Elevation of the top of the practice	← yes

If a surface sand filter is proposed:

YES	ac	Drainage Area check.	← < 10 ac
10,796	cf	V = volume of storage[4, 5] (attach a stage-storage table)	← ≥ 75%WQV
18.0	inches	D_{FC} = filter course thickness	← 18"
Sheet	C-10.2	Note what sheet in the plan set contains the filter course specification	
NA	Yes/No	Access grate provided?	← yes
Crushed Stone		The filter shall not be covered in grass. What is covering the filter?	

If an underground sand filter is proposed:

YES	ac	Drainage Area check.	← < 10 ac
	cf	V = volume of storage[4, 5] (attach a stage-storage table)	← ≥ 75%WQV
	inches	D_{FC} = filter course thickness	← 24"
Sheet		Note what sheet in the plan set contains the filter course specification	
	Yes/No	Access grate provided?	← yes

If a bioretention area is proposed:

NO	ac	Drainage Area no larger than 5 ac?	← yes
	cf	V = volume of storage[4, 5] (attach a stage-storage table)	← ≥ WQV
	inches	D_{FC} = filter course thickness	← 18"
Sheet		Note what sheet in the plan set contains the filter course specification	
	:1	Pond side slopes	← ≥2:1
Sheet		Note what sheet in the plan set contains the planting plans and surface cover	

If porous pavement is proposed:

		Type of pavement proposed (concrete? Asphalt? Pavers? Etc)	
	sf	A_{SA} = surface area of the pervious pavement	
-	:1	ratio of the contributing area to the pervious surface area	← 5:1
	inches	D_{FC} = filter course thickness	← 12"
Sheet		Note what sheet in the plan set contains the filter course spec.	← 304.1 sand

1. If the practice is a tree box filter, the drainage area shall be < 0.1 acre

2. Rate of the limiting layer (either the filter course or the underlying soil). See Vol. 2 of the NH Stormwater Manual, Ch. 2-4, for guidance on determining the infiltration rate.

3. If not within a GPA or WSIPA: SHWT/Bedrock must be at least 1 foot below the filter course material (or an underdrain must drain the SHWT to at least one foot below the filter course material). If within a GPA or WSIPA: SHWT must be at least two feet below the bottom of the practice OR the filter course material must be at least twice as thick as required and the SHWT must be at least one foot below the filter course material.

4. Volume without depending on infiltration. The storage above the filter media shall not include the volume above the outlet structure, if any.

5. The volume includes the storage above the filter but below the invert of the outlet structure (if any), the filter media voids, and the pretreatment area.

Designer's Notes:

The entire water quality volume is being filtered and outleted by the underdrain.

See HydroCAD printouts for stage-storage chart/table

Project #: 52036.00
Project: Wind Farm
Location: Groton, New Hampshire
Calculated by: B. Anderson Date: 11/9/2011
Checked by: Date:
Title: Culvert Sizing Summary

Culvert	Location	Station	Size	Material	Design Storm	Flow (CFS)	Drainage Area (Ac)	Comments
100	Groton Hollow Road	59+00	12"	CPP	25-year	0.3	0.2	
101	Groton Hollow Road	61+46	18"	CPP	25-year	10.5	10.3	Shifted location to accommodate new O&M bldg. layout
102	Groton Hollow Road	67+31	**49"x33" ***	CMP Arch	50-year	38.5	30.0	
103	Groton Hollow Road	68+56	35"x24"	CMP Arch	25-year	15.1	13.9	
104	Groton Hollow Road	73+44	35"x24"	CMP Arch	25-year	13.9	13.6	
105	Groton Hollow Road	78+55	18"	CPP	25-year	9.3	7.1	
106	Groton Hollow Road	85+04	15"	CPP	25-year	4.1	2.9	
107	Groton Hollow Road	87+85	18"	CPP	25-year	6.4	5.3	
108	Groton Hollow Road	89+71	18"	CPP	25-year	5.7	5.5	
109	Groton Hollow Road	95+38	42"x29"	CMP Arch	50-year	43.5	26.5	
110	Groton Hollow Road	100+20	18"	CPP	25-year	6.9	6.5	
111	Groton Hollow Road	107+02	28"x20"	CMP Arch	25-year	3.6	3.5	
112	Groton Hollow Road	109+30	9'x2'	Steel Box	50-year	58.8	47.5	9'x3' size corrected to reflect calculations and plans.
113	Groton Hollow Road	126+45	12"	CPP	25-year	0.8	0.6	
114	Groton Hollow Road	127+78	9'x2'	Steel Box	50-year	64.1	57.2	9'x3' size corrected to reflect calculations and plans.
114A	Groton Hollow Road	135+08	15"	CPP	25-year	2.0	1.4	
115	Groton Hollow Road	137+65	35"x24"	CMP Arch	50-year	27.4	22.2	
116	Groton Hollow Road	142+31	**83"x57" ***	CMP Arch	50-year	161.4	110.6	
117	Groton Hollow Road	143+11	18"	CPP	25-year	6.4	5.8	
118	Groton Hollow Road	148+71	12"	CPP	25-year	1.6	1.0	
119	Groton Hollow Road	150+89	12"	CPP	25-year	1.5	1.0	
217A	Groton Hollow Road	178+00	15"	CPP	25-year	1.2	0.5	Added additional culvert along road
218	Groton Hollow Road	167+61	Ex. 6'x9'	Stone Box	NA	NA	562.4	
219	Groton Hollow Road	163+23	9'x3'	Steel Box	50-year	105.1	81.9	
220	Groton Hollow Road	157+72	21"x15"	CMP Arch	25-year	11.5	16.8	
221	Groton Hollow Road	154+15	9'x2'	Steel Box	50-year	101.5	125.6	9'x3' size corrected to reflect calculations and plans.
222	Groton Hollow Road	±169+00	18'x3.5'	Steel Box	100-year	423.1	298.4	17.3'x3.8' size revised, revised drainage area/flow
201	East Access Road	82+91	17"x13"	CMP Arch	25-year	6.5	7.0	
202	East Access Road	67+37	24"x18"	CMP Arch	25-year	8.5	13.8	Revised drainage area/flow
202A	East Access Road	72+50	15"	CMP	25-year	3.7	4.5	Replaced stone mattress with culvert

Project #: 52036.00
Project: Wind Farm
Location: Groton, New Hampshire
Calculated by: B. Anderson Date: 11/9/2011
Checked by: Date:
Title: Culvert Sizing Summary

Culvert	Location	Station	Size	Material	Design Storm	Flow (CFS)	Drainage Area (Ac)	Comments
202B	East Access Road	76+55	15"	CMP	25-year	1.3	3.1	Replaced stone mattress with culvert
203	East Access Road	60+50	57"x38"	CMP Arch	50-year	43.6	45.6	Revised drainage area/flow
204	East Access Road	59+87	24"x18"	CMP Arch	25-year	3.9	3.2	
205	East Access Road	53+96	42"x29"	CMP Arch	50-year	49.3	36.2	Revised drainage area/flow
206	East Access Road	53+56	24"x18"	CMP Arch	25-year	22.2	16.0	
207	East Access Road	47+23	24"x18"	CMP Arch	25-year	8.5	5.3	Revised drainage area/flow
208	East Access Road	41+67	**64"x43" ***	CMP Arch	50-year	66.5	54.9	Revised drainage area/flow
208A	East Access Road	35+52	15"	CMP	25-year	2.1	1.4	Replaced stone mattress with culvert
209	East Access Road	32+29	**71"x47" ***	CMP Arch	50-year	71.0	58.7	
210	East Access Road	31+88	24"x18"	CMP Arch	25-year	1.2	0.8	Revised drainage area/flow
211	East Access Road	30+69	24"x18"	CMP Arch	25-year	1.3	1.0	
212	East Access Road	26+92	**35"x24" ***	CMP Arch	25-year	12.5	11.4	Increased 28"x20" culvert size to lower HW depth.
213	East Access Road	26+12	**64"x43" ***	CMP Arch	50-year	85.7	67.4	
213A	East Access Road	22+86	15"	CMP	25-year	1.4	1.0	Replaced stone mattress with culvert
214	East Access Road	20+74	24"x18"	CMP Arch	25-year	3.2	2.4	
214A	East Access Road	15+09	24"x18"	CMP Arch	25-year	6.0	5.0	Replaced stone mattress with culvert
215	East Access Road	13+15	24"	RCP	25-year	10.4	5.2	
216	East Access Road	11+41	24"x18"	CMP Arch	25-year	2.8	2.2	
217	East Access Road	6+24	18'x3.5'	Steel Box	100-year	364.0	251.3	17.3'x3.8' size revised, revised drainage area/flow
201A	Northeast Access Rd.	2+32	18"	CMP	25-year	9.1	12.4	Replaced stone mattress with culvert (at low point)
201B	Northeast Access Rd.	20+60	24"	CMP	25-year	2.2	1.4	Added culvert at low point (new road alignment)
501	Southeast Access Rd.	30+88	42"x29"	CMP Arch	50-year	33.4	23.4	
502	Southeast Access Rd.	37+64	17"x13"	CMP Arch	25-year	3.2	2.6	
503	Southeast Access Rd.	38+58	17"x13"	CMP Arch	25-year	4.3	3.4	
504	Southeast Access Rd.	39+55	24"x18"	CMP Arch	25-year	9.7	6.4	
505	Southeast Access Rd.	41+00	17"x13"	CMP Arch	25-year	5.5	2.6	
601	West Access Road	9+47	15"	CPP	25-year	2.2	3.4	Replaced stone mattress with culvert
601A	West Access Road	15+9	24"	CPP	25-year	14.4	10.7	Replaced stone mattress with culvert
602	West Access Road	19+29	21"x15"	CMP Arch	25-year	7.7	7.3	
603	West Access Road	20+76	17"x13"	CMP Arch	25-year	2.4	1.7	

Project #: 52036.00
Project: Wind Farm
Location: Groton, New Hampshire
Calculated by: B. Anderson Date: 11/9/2011
Checked by: Date:
Title: Culvert Sizing Summary

Culvert	Location	Station	Size	Material	Design Storm	Flow (CFS)	Drainage Area (Ac)	Comments
604	West Access Road	21+35	**57"x38" ***	CMP Arch	50-year	100.0	62.7	Revised drainage area/flow
605	West Access Road	23+84	35"x24"	CMP Arch	25-year	10.1	11.3	Revised drainage area/flow
605A	West Access Road	27+00	15"	CPP	25-year	0.6	3.6	Replaced stone mattress with culvert
606	West Access Road	31+51	12"	CPP	25-year	4.2	8.2	Replaced stone mattress with culvert
607	West Access Road	39+43	12"	CPP	25-year	2.8	2.8	Replaced stone mattress with culvert
607A	West Access Road	44+25	15"	CPP	25-year	2.4	1.2	Replaced stone mattress with culvert (at low point)
701	West Ridge Road	51+56	**49"x33" ***	CMP Arch	50-year	33.1	24.7	Revised drainage area/flow
701A	West Ridge Road	57+50	15"	CPP	25-year	1.9	2.1	Replaced stone mattress with culvert
702	West Ridge Road	62+13	17"x13"	CMP Arch	25-year	2.2	2.0	
702A	West Ridge Road	3+75	12"	CPP	25-year	5.8	6.6	Replaced stone mattress with culvert (at low point)
702B	West Ridge Road	64+64	12"	CPP	25-year	6.4	7.0	Replaced stone mattress with culvert (at low point)
703	West Ridge Road	5+16	17"x13"	CMP Arch	25-year	2.1	1.9	Added culvert due to regrading of turbine pad
704	West Ridge Road	69+29	21"x15"	CMP Arch	25-year	10.0	4.9	Replaced stone mattress with culvert
705	West Ridge Road	78+15	18"	CPP	25-year	7.3	4.1	Replaced stone mattress with culvert (at low point)
706	West Ridge Road	80+25	15"	CPP	25-year	2.8	3.8	Replaced stone mattress with culvert
707	West Ridge Road	111+75	12"	CPP	25-year	2.9	1.6	Replaced stone mattress with culvert (at low point)
708	West Ridge Road	101+17	12"	CPP	25-year	2.9	4.7	Replaced stone mattress with culvert (at low point)
801	North Access Road	4+42	**49"x33" ***	CMP Arch	50-year	47.5	33.1	35"x24" size increased, revised drainage area/flow
801A	North Access Road	10+30	15"	CMP	25-year	2.0	1.4	Replaced stone mattress with culvert
802	North Access Road	13+50	17"x13"	CMP Arch	25-year	8.9	6.7	
802A	North Access Road	18+07	17"x13"	CMP Arch	25-year	3.2	2.1	Replaced stone mattress with culvert
803	North Access Road	21+42	24"x18"	CMP Arch	25-year	14.3	15.3	17"x13" size increased, revised drainage area/flow
803A	North Access Road	25+50	18"	CMP	25-year	2.5	1.5	Replaced stone mattress with culvert
804	North Access Road	29.88	**49"x33" ***	CMP Arch	50-year	46.6	34.1	Revised drainage area/flow
804A	North Access Road	31+85	15"	CMP	25-year	4.3	4.0	Replaced stone mattress with culvert
804B	North Access Road	38+35	18"	CMP	25-year	9.5	17.6	Replaced stone mattress with culvert
805	North Access Road	43+53	21"x15"	CMP Arch	25-year	10.2	13.0	17"x13" size increased, revised drainage area/flow
806	North Access Road	45+41	12"	CPP	25-year	4.0	5.2	Revised drainage area/flow
807	North Access Road	46+42	28"x20"	CMP Arch	50-year	28.1	20.5	
808	North Access Road	46+90	17"x13"	CMP Arch	25-year	1.2	2.4	Revised drainage area/flow

Project #: 52036.00
Project: Wind Farm
Location: Groton, New Hampshire
Calculated by: B. Anderson Date: 11/9/2011
Checked by: Date:
Title: Culvert Sizing Summary

Culvert	Location	Station	Size	Material	Design Storm	Flow (CFS)	Drainage Area (Ac)	Comments
809	North Access Road	48+80	**49"x33" ***	CMP Arch	50-year	49.5	30.6	42"x29" size increased to reduce HW depth
810	North Access Road	48+50	12"	CMP	25-year	2.2	2.3	Added cluvert under ex. road/path
811	North Access Road	53+35	15"	CMP	25-year	4.5	6.1	Replaced stone mattress with culvert
901	North Ridge Road	72+55	12"	CPP	25-year	7.0	8.8	Revised drainage area/flow
902	North Ridge Road	79+16	12"	CPP	25-year	0.8	0.7	
902	North Ridge Road	79+00	12"	CPP	25-year	1.3	4.2	
902A	North Ridge Road	77+13	15"	CPP	25-year	6.9	8.3	
903	North Ridge Road	83+50	24"x18"	CMP Arch	25-year	7.3	13.5	Revised drainage area/flow
903A	North Ridge Road	88+12	15"	CPP	25-year	3.4	7.3	Replaced stone mattress with culvert
904	North Ridge Road	110+50	12"	CPP	25-year	2.6	3.0	Replaced stone mattress with culvert (at low point)

* Pipe calculation and sizing based on the next smaller nominal pipe size than currently shown. Perennial Stream crossings shall have pipe dimensions as shown on this chart with 8" of gravel placed within it to act as a natural bottom for the stream

Shaded cells indicate revisions from the permit plans and shaded rows indicate culvert additions to the permit plans.

(2)

Clayton, Michael

From: Walker, Peter <PWalker@VHB.com>
Sent: Thursday, November 03, 2011 3:40 PM
To: crennie@des.state.nh.us
Cc: Emmett, Doren; Clayton, Michael; Leo, Michael
Subject: Meeting Request - Groton Wind Farm

Hi Craig –

I just left you a voicemail.

We will be submitting some revised plans to the Department in the next couple days for the Groton Wind Farm. We'd like to come by to introduce you to a few of the construction personnel and look through the plans. Right now, folks are somewhat flexible, but Tuesday afternoon or anytime Thursday would work best for us. Let us know what works for you.

Thanks,
Pete

Peter J. Walker
Director, Environmental Services

VHB | Vanasse Hangen Brustlin, Inc.
Transportation | Land Development | Environmental Services

6 Bedford Farms Drive, Suite 607
Bedford, NH 03110-6532
Phone: 603.644.0888 x2542 | Fax: 603.644.2385
Mobile: 603.303.1038
pwalker@vhb.com

www.vhb.com

Clayton, Michael

From: Rennie, Craig <Craig.Rennie@des.nh.gov>
Sent: Monday, November 07, 2011 2:49 PM
To: Walker, Peter
Cc: Emmett, Doren; Clayton, Michael; Leo, Michael
Subject: RE: Meeting Request - Groton Wind Farm

Thursday works best for me. How about 9:00 am on the 10th in the DES lobby?
Thanks,
Craig

~~~~~~~~~~~~~~~~~~~~

Craig D. Rennie, CWS, CWB
Land Resource Specialist
NH Department of Environmental Services
Water Division
29 Hazen Drive, PO Box 95
Concord, NH 03302-0095
ph: (603) 271-0676 fax: (603) 271-6588
email: craig.rennie@des.nh.gov

-----Original Message-----
From: Walker, Peter [mailto:PWalker@VHB.com]
Sent: Thursday, November 03, 2011 3:40 PM
To: Rennie, Craig
Cc: Emmett, Doren; 'Clayton, Michael'; Leo, Michael
Subject: Meeting Request - Groton Wind Farm

Hi Craig –

I just left you a voicemail.

We will be submitting some revised plans to the Department in the next couple days for the Groton Wind Farm. We'd like to come by to introduce you to a few of the construction personnel and look through the plans. Right now, folks are somewhat flexible, but Tuesday afternoon or anytime Thursday would work best for us. Let us know what works for you.

Thanks,
Pete

Peter J. Walker
Director, Environmental Services
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6 Bedford Farms Drive, Suite 607
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Phone: 603.644.0888 x2542 | Fax: 603.644.2385
Mobile: 603.303.1038
pwalker@vhb.com
www.vhb.com
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(3)

Clayton, Michael

Subject:	Groton Wind - DES Meeting
Location:	DES Office
Start:	Thu 11/10/2011 1:00 PM
End:	Thu 11/10/2011 2:00 PM
Show Time As:	Tentative
Recurrence:	(none)
Meeting Status:	Not yet responded
Organizer:	Emmett, Doren
Required Attendees:	Rennie, Craig [Craig.Rennie@des.nh.gov]; Clayton, Michael; Leo, Michael
Optional Attendees:	Cherian, Ed; Varughese, Jebby; Revell, Kelly

When: Thursday, November 10, 2011 1:00 PM-2:00 PM (GMT-05:00) Eastern Time (US & Canada).
Where: DES Office

Note: The GMT offset above does not reflect daylight saving time adjustments.

~~*~*~*~*~*~*~*~*~*

Meet with Craig Rennie in DES Lobby located at:

NH Department of Environmental Services
Water Division
29 Hazen Drive, PO Box 95
Concord, NH 03302-0095
ph: (603) 271-0676 fax: (603) 271-6588
email: craig.rennie@des.nh.gov

Clayton, Michael

Subject:	Groton Wind - Construction Plan Submittal
Location:	NHDES, 29 Hazen Drive, Concord, NH
Start:	Thu 11/10/2011 10:00 AM
End:	Thu 11/10/2011 11:30 AM
Show Time As:	Tentative
Recurrence:	(none)
Meeting Status:	Not yet responded
Organizer:	Walker, Peter

When: Thursday, November 10, 2011 10:00 AM-11:30 AM (GMT-05:00) Eastern Time (US & Canada).
Where: NHDES, 29 Hazen Drive, Concord, NH

Note: The GMT offset above does not reflect daylight saving time adjustments.

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To confirm our meeting, Pete, Doren and Mike will meet with Craig this Thursday at 10 AM in Concord.

(4)

Clayton, Michael

From: Rennie, Craig <Craig.Rennie@des.nh.gov>
Sent: Monday, December 05, 2011 1:49 PM
To: Walker, Peter
Cc: Emmett, Doren; Cherian, Ed; Kelly.Revell@iberdrolaren.com; Wilkes, Kristopher; Clayton, Michael
Subject: Revised Plans/Amended Permits for Groton Wind Farm, Wetland File #2010-00745, AoT File #100325-033
Attachments: GROTON WIND POWER_WETLAND AMENDMENT_CDR.pdf; AoT_GROTON WIND_AMENDED_CDR.pdf

Peter,
The Department of Environmental Services (DES) has reviewed the revised grading and wetland impact plans (showing minor roadway modifications, relocation of the O&M building, minor transmission line relocations, and a reduction in stone mattresses) that further reduce the overall disturbance by 12 acres, and increases the wetland impact by 280 square feet, and has determined that these minor modifications are acceptable as presented. Attached please find the amended approvals for the Wetland and Alteration of Terrain Permits. The attached amended permits shall serve as confirmation to proceed with the minor modifications as depicted on the revised plans by VHB dated October 28, 2011. Please call if you have any questions.
Thanks,
Craig

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Craig D. Rennie, CWS, CWB
Land Resource Specialist
NH Department of Environmental Services
Water Division
29 Hazen Drive, PO Box 95
Concord, NH 03302-0095
ph: (603) 271-0676 fax: (603) 271-6588
email: craig.rennie@des.nh.gov
~~~~~~~~~~~~~~~~~~~~

WETLANDS BUREAU PERMIT AMENDMENT
December 5, 2011 (Original decision issued Oct. 8, 2010)

RECOMMEND APPROVAL WITH THE FOLLOWING PERMIT CONDITIONS:

PROPOSED AMENDMENT:

Amend permit to allow minor plan revisions that includes roadway modifications, relocation of the O&M building, minor transmission line relocations, and a reduction in stone mattresses. The revised plans show a 12 acre reduction in the total earth disturbance, and a minor increase of wetland impact by 280 square feet.

AMEND PROJECT DESCRIPTION:

Dredge and fill 1.65 acres (71,960 square feet) of wetlands and streams (impacting 4,302 linear feet) and temporarily impact .33 acres (14,130 square feet) of wetlands and 320 square feet within a stream, to construct a power generating wind park that will include the construction of 24 wind turbines (2.0 megawatts each), approximately 12 miles of gravel access drives, a 4,000 square foot operations/maintenance building, stockpile and lay down pad areas, and associated transmission lines. Mitigate impacts by making a in-lieu fee payment of $150,000 into the DES Aquatic Resources Mitigation (ARM) Fund; by upgrading nine existing stream crossings along Groton Hollow Road to meet the new DES stream rules; and by providing technical assistance to the Society for the Protection of New Hampshire Forests (SPNHF) by donating survey data, title research, and environmental baseline data to assist SPNHF in their efforts to protect 6,578 acres of undeveloped land known as the Green Acre Woodlands Project.

PROJECT SPECIFIC CONDITIONS:

1. All work shall be in accordance with revised plans by Vanasse Hangen Brustlin, Inc. dated October 28, 2011, as received by the NH Department of Environmental Services (DES) on November 10, 2011.
2. Prior to construction, any plan revisions or changes in construction details or sequences shall be submitted to DES for review and approval.
3. Any further alteration of areas on this property that are within the jurisdiction of the DES Wetlands Bureau will require a new application and further permitting by the Bureau.
4. This permit is contingent on approval by the DES Alteration of Terrain Bureau.
5. No construction activities shall occur on the project after expiration of the approval unless the approval has been extended by the New Hampshire Energy Facility Site Evaluation Committee (SEC).
6. Appropriate siltation/erosion/turbidity controls shall be in place prior to construction, shall be maintained during construction, and remain in place until the area is stabilized. Silt fence(s) must be removed once the area is stabilized.
7. Discharge from dewatering of work areas shall be to sediment basins that are: a) located in uplands; b) lined with hay bales or other acceptable sediment trapping liners; c) set back as far as possible from wetlands and surface waters, in all cases with a minimum of 20 feet of undisturbed vegetated buffer.
8. Dredged material shall be placed outside of the jurisdiction of the DES Wetlands Bureau.
9. Stream work shall be done during low flow conditions.

10. Culvert outlets shall be protected in accordance with the DES Best Management Practices for Urban Stormwater Runoff Manual (January 1996) and the Stormwater Management and Erosion and Sediment Control Handbook for Urban and Developing Areas in New Hampshire (August 1992).
11. Proper headwalls shall be constructed within seven days of culvert installation.
12. Within three days of final grading, all exposed soil areas shall be stabilized by seeding and mulching during the growing season, or if not within the growing season, by mulching with tack or netting and pinning on slopes steeper than 3:1.
13. Where construction activities have been temporarily suspended within the growing season, all exposed soil areas shall be stabilized within 14 days by seeding and mulching.
14. Where construction activities have been temporarily suspended outside the growing season, all exposed areas shall be stabilized within 14 days by mulching and tack. Slopes steeper than 3:1 shall be stabilized by matting and pinning.
15. The contractor responsible for completion of the work shall utilize techniques described in the New Hampshire Stormwater Manual, Volume 3, Erosion and Sediment Controls During Construction (December 2008).

Restoration Conditions:

16. This permit is contingent upon the restoration of 14,450 square feet of wetlands and streams that are being temporarily impacted in accordance with the plans received by DES on March 29, 2010.
17. All temporary wetland and stream impact areas shall be properly restored, and shall be monitored to ensure that functioning wetland areas similar to those destroyed by the project are replicated. Remedial measures may be necessary for successful restoration, which can include replanting, relocating plantings, removal of invasive species, changing soil composition and depth, changing the elevation of the wetland surface, and changing the hydrologic regime.
18. The permittee shall designate a qualified professional who will be responsible for monitoring and ensuring that the restoration areas are completed in accordance with the plans. Monitoring shall be accomplished in a timely fashion and remedial measures taken if necessary. The Wetlands Bureau shall be notified in writing of the designated professional prior to the start of work and if there is a change of status during the project.
19. The permittee or a designee shall conduct a follow-up inspection after the first growing season, to review the success of the restoration areas and schedule remedial actions if necessary. A report outlining these follow-up measures and a schedule for completing the remedial work shall be submitted by December 1 of that year. Similar inspections, reports and remedial actions shall be undertaken in at least the second (2^{nd}) year following the completion of each restoration area.
20. Wetland restoration areas shall have at least 75% successful establishment of wetlands vegetation after a full growing season, or shall be replanted and re-established until a functional wetland is replicated in a manner satisfactory to the DES Wetlands Bureau.
21. The permittee shall attempt to control invasive, weedy species such as purple loosestrife (Lythrum salicaria) and common reed (Phragmites australis) by measures agreed upon by the Wetlands Bureau if the species is found in the restoration areas during construction and during the early stages of vegetative establishment.
22. A post-construction report documenting the status of the completed project with photographs shall be submitted to the Wetlands Bureau within 60 days of the completion of construction.

Mitigation Conditions:

23. This approval is contingent on receipt by DES of a one time payment of $150,000 to the DES Aquatic Resource Mitigation (ARM) Fund. If the project is approved by the New Hampshire Energy Facility Site Evaluation Committee (SEC), then the payment shall be received by DES within 120 days of the date of their approval.
24. This permit is contingent upon the upgrade of nine existing stream crossings along Groton Hollow Road in order to meet the standards of the DES stream rules (Env-Wt 900).
25. This permit is contingent upon Groton Wind, LLC donating to SPNHF the property survey data and mapping, title research, and environmental baseline data to support their efforts in preserving 6,578 acres of undeveloped land known as the Green Acre Woodlands Project.

FINDINGS:

1. This project is classified as a Major Impact Project per NH Administrative Rule Env-Wt 303.02(c), as wetland impacts are greater than 20,000 square feet.
2. The need for the proposed impacts has been demonstrated by the applicant per Rule Env-Wt 302.01.
3. The applicant has provided evidence which demonstrates that this proposal is the alternative with the least adverse impact to areas and environments under the department's jurisdiction per Rule Env-Wt 302.03.
4. The applicant has demonstrated by plan and example that each factor listed in Rule Env-Wt 302.04(a), Requirements for Application Evaluation, has been considered in the design of the project.
5. DES Staff conducted a field inspection of the proposed project on June 29, 2010. Field inspection determined that the majority of the site has been historically and actively logged and that the upgrades to culverts along Groton Hollow Road were necessary in order to meet the stream rules.
6. Public hearing is not required with the finding that the project will not impact wetland areas that are considered to be of special value from a local, regional, or state perspective pursuant to Rule Env-Wt 101.90
7. The applicant has reviewed on-site options for mitigation and the department has determined that this project is acceptable for payment to the Aquatic Resource Mitigation (ARM) Fund.
8. The payment calculated for the proposed wetland loss equals $150,000.
9. The Department decision is issued in letter form and upon receipt of the ARM fund payment, the Department shall issue a posting permit in accordance with Env-Wt 803.08(f).
10. The payment into the ARM fund shall be deposited in the DES fund for the Pemigewasset River Watershed per RSA 482-A:29.

ALTERATION OF TERRAIN (AoT) BUREAU PERMIT AMENDMENT
December 5, 2011 (Original decision issued Oct. 8, 2010)

PROPOSED AMENDMENT:
Amend permit to allow minor plan revisions that includes roadway modifications, relocation of the O&M building, minor transmission line relocations, and a reduction in stone mattresses. The revised plans show a 12 acre reduction in the total disturbance, and a minor increase of wetland impact by 280 square feet.

RECOMMEND APPROVAL WITH THE FOLLOWING PERMIT CONDITIONS:
(Approval includes permit conditions from the Watershed Management Bureau (WMB) to satisfy 401 Water Quality Certification concerns, and from the Drinking Water and Groundwater Bureau (DWGB) to satisfy concerns regarding ledge blasting and monitoring Best Management Practices)

AMENDED PROJECT DESCRIPTION:
Construct a power generating wind park that will include the construction of 24 wind turbines (2.0 megawatts each), approximately 12 miles of gravel access drives, a 4,000 square foot operations and maintenance building, stockpile and lay down pad areas, and associated transmission lines. The total area of contiguous disturbance has been calculated to be 103.6 acres (4,512,834 square feet).

PROJECT SPECIFIC CONDITIONS:
1. Activities shall not cause or contribute to any violations of the surface water quality standards established in Administrative Rule Env-Wq 1700.
2. Revised plans shall be submitted for an amendment approval prior to any changes in construction details or sequences. The Department must be notified in writing within ten days of a change in ownership.
3. The Department must be notified in writing prior to the start of construction and upon completion of construction. Forms are available at: http://des.nh.gov/organization/divisions/water/aot/categories/forms.htm.
4. The revised plans dated October 28, 2011 and supporting documentation in the file are a part of this approval.
5. No construction activities shall occur on the project after expiration of the approval unless the approval has been extended by the New Hampshire Energy Facility Site Evaluation Committee (SEC).
6. This permit does not relieve the Applicant from the obligation to obtain other local, state or federal permits that may be required (e.g., from US EPA, US Army Corps of Engineers, etc.). Projects disturbing over 1 acre may require a federal stormwater permit from EPA. Information regarding this permitting process can be obtained at: http://des.nh.gov/organization/divisions/water/stormwater/construction.htm.
7. The smallest practical area shall be disturbed during construction activities.
8. The Applicant shall employ the services of an environmental monitor ("Monitor"). The Monitor shall be a Certified Professional in Erosion and Sediment Control or a Professional Engineer licensed in the State of New Hampshire and shall be employed to inspect the site from the start of alteration of terrain activities until the alteration of terrain activities are completed.

9. The Monitor shall provide technical assistance and recommendations to the Contractor on the appropriate Best Management Practices for Erosion and Sediment Controls required to meet the requirements of RSA 485-A:17 and all applicable DES permit conditions.
10. Prior to beginning construction, the contractor's name, address, and phone number shall be submitted to DES via email (to Denise Frappier at denise.frappier@des.nh.gov and to Craig Rennie at: craig.rennie@des.nh.gov).
11. Unless otherwise authorized by DES, the Applicant shall keep a sufficient quantity of erosion control supplies on the site at all times during construction to facilitate an expeditious (i.e., within 24 hour) response to any construction related erosion issues on the site.
12. The Applicant shall develop and submit a Construction BMP Inspection and Maintenance Plan to DES for approval at least 90 days prior to construction. Unless otherwise authorized by DES, the plan shall incorporate all elements described in **Appendix A** (items A through J). The Applicant shall then implement the approved plan.
13. The Applicant shall prepare a turbidity sampling plan as specified in **Appendix A** of this permit. The plan shall be submitted to DES for approval at least 90 days prior to construction. The Applicant shall then implement the approved plan. Unless otherwise authorized by DES, the turbidity sampling results along with station ID, date, time, other field notes, and a description of corrective actions taken when violations of state surface water quality criteria for turbidity are found, shall be submitted to DES via electronic mail within 48 hours of collection
14. The Applicant shall prepare and submit a Spill Prevention, Control, and Countermeasures plan (SPCC) for the Activity in accordance with federal regulations (40 CFR part 112). The plan shall include a certification by a Professional Engineer licensed in the State of New Hampshire. The Applicant shall submit the plan to DES Watershed Management Bureau for review and approval at least 90 days prior to the installation of the first turbine. The SPCC Plan shall include, but not be limited to, operating procedures to prevent oil spills, control measures installed to prevent oil from entering surface waters, countermeasures to contain, clean-up and mitigate the effects of an oil spill, and facility inspections. The Applicant shall then implement the approved plan and maintain records demonstrating compliance with the plan. Such records shall be made available to DES within 30 days of receiving a written request by DES.
15. The Applicant shall submit a plan to prevent water quality violations due to discharges of concrete wash water during construction. The Applicant shall submit the plan to DES Watershed Management Bureau for review and approval at least 90 days prior to placement of any concrete within the Activity area. The Applicant shall then implement the approved plan.
16. As proposed by the Applicant, unless otherwise authorized by DES, herbicides and pesticides shall not be used on the site for the construction or operation of the Activity.
17. Unless otherwise authorized by DES, fertilizers shall only be applied once on soils disturbed during construction to support the initial establishment of vegetation. Prior to fertilizer application, soils shall be tested to determine the minimum amounts of lime, nitrogen (N), phosphorus (P) and potassium (K) needed to support vegetation. Lime application rates, fertilizer selection (in terms of N, P and K content) and fertilizer application rates shall be consistent with the soil test results. Fertilizers shall not contain any pesticides. Where possible, fertilizer with slow release nitrogen shall be used. Soil test results, the name, brand and nutrient content (N, P and K) of fertilizer and application rates for lime and fertilizer shall be provided to DES within 30 days of receiving a request from DES. As proposed by the Applicant, unless otherwise authorized by DES, no fertilizers shall be used for the Activity following construction.

18. As proposed by the Applicant, unless otherwise authorized by DES, no de-icing agents (including use of sands containing chloride) shall be used on the Activity either during construction or once the Activity is in operation.
19. Unless otherwise authorized by DES, the Applicant shall limit forest clearing within a 50-foot buffer of Clark Brook to 0.2 acres (<1% change from pre-Activity conditions) and within a 50-foot buffer of all perennial streams to 3.6 acres (5% change from pre-Activity conditions).
20. Unless otherwise authorized by DES, the Applicant shall develop and submit a monitoring plan to DES for approval at least 90 days prior to construction. The purpose of the plan is to confirm that operation of the Activity is not causing or contributing to violations of state surface water quality standards. The plan shall include the parameters to be sampled, the location, timing and frequency of sampling, sampling and laboratory protocols, quality assurance/quality control provisions as well as when data will be submitted to DES. The Applicant shall consult with DES and submit the monitoring data in a format that can be automatically uploaded into the DES Environmental Database. Once approved by DES, the Applicant shall implement the sampling plan.
21. The Applicant shall identify drinking water wells located within 2000 feet of the proposed blasting activities. Develop and implement a groundwater quality sampling program to monitor for nitrate and nitrite either in the drinking water supply wells or in other wells that are representative of the drinking water supply wells in the area. The program must be approved by the DES DWGB.
22. The following Best Management Procedures for blasting shall be complied with:
    (1) Loading practices. The following blasthole loading practices to minimize environmental effects shall be followed:
        a) Drilling logs shall be maintained by the driller and communicated directly to the blaster. The logs shall indicate depths and lengths of voids, cavities, and fault zones or other weak zones encountered as well as groundwater conditions.
        b) Explosive products shall be managed on-site so that they are either used in the borehole, returned to the delivery vehicle, or placed in secure containers for off-site disposal.
        c) Spillage around the borehole shall either be placed in the borehole or cleaned up and returned to an appropriate vehicle for handling or placement in secured containers for off-site disposal.
        d) Loaded explosives shall be detonated as soon as possible and shall not be left in the blastholes overnight, unless weather or other safety concerns reasonably dictate that detonation should be postponed.
        e) Loading equipment shall be cleaned in an area where wastewater can be properly contained and handled in a manner that prevents release of contaminants to the environment.
        f) Explosives shall be loaded to maintain good continuity in the column load to promote complete detonation. Industry accepted loading practices for priming, stemming, decking and column rise need to be attended to.
    (2) Explosive Selection. The following BMPs shall be followed to reduce the potential for groundwater contamination when explosives are used:
        a) Explosive products shall be selected that are appropriate for site conditions and safe blast execution.
        b) Explosive products shall be selected that have the appropriate water resistance for the site conditions present to minimize the potential for hazardous effect of the product upon groundwater.

(3) Prevention of Misfires. Appropriate practices shall be developed and implemented to prevent misfires.

(4) Muck Pile Management. Muck piles (the blasted pieces of rock) and rock piles shall be managed in a manner to reduce the potential for contamination by implementing the following measures:
   a) Remove the muck pile from the blast area as soon as reasonably possible.
   b) Manage the interaction of blasted rock piles and stormwater to prevent contamination of water supply wells or surface water.

(5) Spill Prevention Measures and Spill Mitigation. Spill prevention and spill mitigation measures shall be implemented to prevent the release of fuel and other related substances to the environment. The measures shall include at a minimum:
   a) The fuel storage requirements shall include:
      i. Storage of regulated substances on an impervious surface;
      ii. Secure storage areas against unauthorized entry;
      iii. Label regulated containers clearly and visibly;
      iv. Inspect storage areas weekly;
      v. Cover regulated containers in outside storage areas;
      vi. Wherever possible, keep regulated containers that are stored outside more than 50 feet from surface water and storm drains, 75 feet from private wells, and 400 feet from public wells; and
      vii. Secondary containment is required for containers containing regulated substances stored outside, except for on premise use heating fuel tanks, or aboveground or underground storage tanks otherwise regulated.
   b) The fuel handling requirements shall include:
      i. Except when in use, keep containers containing regulated substances closed and sealed;
      ii. Place drip pans under spigots, valves, and pumps;
      iii. Have spill control and containment equipment readily available in all work areas;
      iv. Use funnels and drip pans when transferring regulated substances; and
      v. Perform transfers of regulated substances over an impervious surface.
   c) The training of on-site employees and the on-site posting of release response information describing what to do in the event of a spill of regulated substances.
   d) Fueling and maintenance of excavation, earthmoving and other construction related equipment will comply with the regulations of the DES. Note these requirements are summarized in "WD-DWGB-22-6 Best Management Practices for Fueling and Maintenance of Excavation and Earthmoving Equipment" or its successor document (see http://des.nh.gov/organization/commissioner/pip/factsheets/dwgb/documents/dwgb-22-6.pdf).

Appendix A:

Details of construction BMP inspection, reporting requirements, and turbidity monitoring

(In light of the sensitive resources within the project area and scale of the proposed activity, the following additional construction BMP inspection and reporting requirements and turbidity monitoring are considered necessary to prevent construction related surface water quality violations)

A. Weekly Erosion Control Meeting: The Applicant's prime Contractor for the Activity (prime Contractor) shall hold weekly erosion control meetings with the Monitor. Minutes of the meeting shall be kept on file and made available to DES upon request.

B. Inspection Frequency: Regular inspections shall be conducted as specified below for the purposes of determining compliance with the permit.
   (1) Daily Inspections: The prime Contractor shall inspect all erosion control measures every day that work is conducted from the time construction commences and earth is disturbed until construction is complete.
   (2) Weekly Inspections: After construction has commenced and earth has been disturbed, the Monitor shall conduct weekly erosion control site inspections to verify all erosion control measures are maintained properly to protect surface waters and wetlands. The Monitor shall document and report its findings, including recommendations for maintenance of BMPs or the addition of new control measures to the prime Contractor.
   (3) Pre-storm inspections: The Monitor shall print the 5-day forecast once daily (7-9 am) for the duration of the project. All forecasts shall be clearly marked with the date and time, kept on file, provided to the prime Contractor. In addition, the 5-day forecast on the day of the weekly meeting shall be attached to the weekly meeting minutes distributed by the Monitor. Inspection shall occur within 24 hours prior to the start of any rain event of 0.5 inches or more in a 24-hour period that is predicted to occur during the workweek. A normal workweek is Monday through Friday. Holidays and weekends are included as part of the normal workweek when work is anticipated to occur on those days. If the predicted event occurs outside of the normal workweek, the inspection shall occur on the normal workday just before any scheduled days off, such as holidays and weekends. Unless otherwise approved by DES, the Accuweather website (http://home.accuweather.com/index.asp?partner=accuweather) shall be used for the purpose of predicting future precipitation amounts. Future precipitation amounts on the Accuweather web site may be determined by typing in the location of the project (city, state and/or zip code), clicking on the link for Days 1-5 forecasts and then clicking on the day(s) of interest.

C. Emergency Inspections During Storm Events: Inspections shall occur during the daylight hours (Monday through Sunday, including holidays) during storm events whenever plumes are visible or if turbidity sampling indicates water quality standards are exceeded due to turbid stormwater from the construction site. Inspections and corrective action shall be implemented during the daylight hours (Monday through Sunday, including holidays) until turbidity water quality standards are met.

D. Post Storm Inspections: Inspections shall occur on the first workday following storms of greater than 0.5 inches in a 24-hour period. Precipitation amounts shall be based on precipitation recorded at a rain gauge installed at the construction site or other approved method. Inspections and corrective action shall be implemented during the daylight hours (Monday through Sunday, including holidays) until turbidity water quality standards are met.

E. Winter Shutdown Inspections: Inspections during winter shut down shall occur as specified in the NPDES General Permit for Stormwater Discharges from Construction Activities (commonly known as the Construction General Permit)]

F. Provisions for Handling Emergencies: Contact information shall be provided to DES for at least two people that DES can contact at any time regarding construction related stormwater concerns. The Applicant shall prepare an Emergency Procedures Plan describing procedures to address and correct emergency, construction related stormwater issues in an expeditious manner. The plan shall include the responsibilities of key individuals, the availability of equipment, and the availability of erosion control and BMP supplies. All emergency erosion control and BMP supplies must be kept on-site.

G. Inspection and Maintenance Plans and Reports: Written inspection and maintenance reports shall include the items stipulated in the EPA NPDES General Permit for Stormwater Discharges from Construction Activities, as well as the predicted 24-hour rainfall for pre-storm inspection reports, measured rainfall amounts for post-inspection reports. The reports shall also indicate if erosion control measures "pass" or "fail" , if the project is being constructed in accordance with the approved sequence, identify any deviation from the conditions of this permit and the approved plans, and identify any other noted deficiencies and include photographic documentation. Unless otherwise authorized by DES, within 24 hours of each inspection, the Monitor shall submit a report with photographic documentation to DES via email (to Denise Frappier at denise.frappier@des.nh.gov and to Craig Rennie at: craig.rennie@des.nh.gov).

H. Weather Station Specifications: Unless otherwise authorized by DES, the Applicant shall be responsible for maintaining a weather station that can measure rainfall to an accuracy of 0.01 inches, monitor temperature to an accuracy of 1 degree Fahrenheit or Celsius, and has hourly data storage and download capabilities.

I. Precipitation Notification Plan: The Applicant shall specify how the Monitor, and others, will be notified when precipitation has occurred that will trigger the need for inspections and/or turbidity sampling. Automatic notification is preferred. If considered necessary and feasible by DES, the weather station shall be equipped to send automatic email notifications to notify the Monitor when construction BMP inspections and/or turbidity sampling is necessary. Should automated email notification be considered necessary, it shall be capable of the following: Start of rain event: Once 0.25 inches of rain or rain-mix precipitation has been measured an automated email notification will be sent to the prime Contractor, the Monitor, and any other interested parties. The email shall provide hourly rainfall, and time of rainfall for the previous 24 hours. End of rain event: Once six hours without rain or rain-mix precipitation has passed an automated email notification will be sent to the prime Contractor, the Monitor and DES. The email shall provide hourly rainfall and time of rainfall from the start of the rain event to the end of the rain event, including the six hour "dry" period.

J. Turbidity Monitoring: To confirm that construction best management practices (BMPs) for controlling erosion are performing as intended, turbidity monitoring is needed. Unless otherwise authorized by DES, the Applicant shall submit a Turbidity Sampling Plan that includes the turbidity monitoring elements specified in the February 2, 2009 DES Inter-Department Communication entitled "Amendment of the November 16, 2006 Guidance for BMP Inspection and Maintenance and Turbidity Sampling and Analysis Plans for I-93 Expansion Project Water Quality Certification". This document includes guidance regarding sampling station number and locations, sampling frequency, sampling duration, size of storms that need to be sampled, how soon after the start of precipitation sampling should begin, quality assurance quality control provisions, and turbidity meter specifications.

(5)

Leo, Michael

From: Rennie, Craig <Craig.Rennie@des.nh.gov>
Sent: Tuesday, February 28, 2012 8:35 AM
To: Leo, Michael
Cc: Walker, Peter
Subject: RE: Groton Wind - NE Road Shift

Mike,
The Department of Environmental Services (DES) Alteration of Terrain Bureau and Wetland Bureau have reviewed the revised plan to shift a portion of the roadway approximately 30 feet from the original location near turbines E-2, E-3 and E-4, and to add a truck turn-around near turbine E-2. DES has determined that these minor modifications are acceptable as presented, and will not require a permit amendment. This email shall serve as confirmation to proceed with the minor modification as depicted on the VHB plan with a revised date of February 13, 2012. Please call if you have any questions.
Thanks,
Craig

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Craig D. Rennie, CWS, CWB
Land Resource Specialist
NH Department of Environmental Services
Water Division
29 Hazen Drive, PO Box 95
Concord, NH 03302-0095
ph: (603) 271-0676 fax: (603) 271-6588
email: craig.rennie@des.nh.gov

-----Original Message-----
From: Leo, Michael [mailto:MLeo@VHB.com]
Sent: Wednesday, February 15, 2012 9:08 AM
To: Rennie, Craig
Cc: Walker, Peter
Subject: Groton Wind - NE Road Shift

Craig,

Attached, for your consideration, are letter and plans describing proposed road adjustment to portion of roadway for Groton Wind Farm Project. AoT Permit No. SEC-0004. The intent is to allow a portion of the roadway to be shifted approximately 30 ft. from previously approved design location.

Please review the attached materials and let me know if we may proceed with the proposed change. Feel free to contact me should you have any questions or require additional information.

Thanks, Mike

Michael J. Leo
VHB | Vanasse Hangen Brustlin, Inc.
Transportation | Land Development | Environmental Services
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